UBC Social Ecological Economic Development Studies (SEEDS) Sustainability Program

Student Research Report

Chancellor Boulevard and East Mall Intersection Redesign

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University of British Columbia

CIVL 446

April 7th, 2025

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UBC Campus and Community Planning The University of British Columbia Vancouver, BC V6T 1Z4



April 7, 2025

Dear UBC Campus and Community Planning Team,

Capstone Consulting Ltd. is pleased to submit our Detailed Design Report for the redesign of the Chancellor Boulevard and East Mall intersection at the University of British Columbia. This report has been prepared as part of the CIVL 446 Capstone Design Course at UBC.

Our goal was to redesign the intersection to better support pedestrian, cyclist, and vehicle safety while creating a gateway to campus. After conducting site visits, technical analyses, and working closely with stakeholders, our team developed a design that addresses current challenges, such as vehicle speeds, poor visibility, and inadequate stormwater management.

The final design features protected active transportation infrastructure, reduced lane widths, bioretention swales, and a signature gateway sign—balancing function, safety, and sustainability within the site constraints. The redesign of the Chancellor Boulevard-East Mall intersection will be a four-legged signalized protected intersection. The intersection includes crosswalks along all intersection legs to allow safer bike and pedestrian crossings. The largest change made to the intersection included condensing the intersection by bringing the south leg eastward, significantly shortening crossings. The intersection provides dedicated sidewalks and bike lanes, providing ample separation between active transportation and traffic. This report outlines our methodology, design rationale, and implementation plan, including risk mitigation strategies and a detailed construction schedule. With consideration of environmental sustainability and stakeholder engagement, our design reflects a comprehensive approach to responsible campus development.

We appreciate the opportunity to work on this meaningful project and hope that our proposed redesign meets your expectations. Please find the full report enclosed, and we look forward to your feedback.

Sincerely,

Team 9 – Capstone Consulting Ltd. CIVL 446 – Capstone Design Project The University of British Columbia



Chancellor Boulevard and East Mall Intersection Redesign

UBC Campus and Community Planning

Final Detailed Design Report

CIVL 446 2024W – Team 9 Capstone Consulting Limited April 7, 2025



EXECUTIVE SUMMARY

The redesign of the current Chancellor Boulevard-East Mall intersection will be a four-legged signalized protected intersection. The intersection includes crosswalks along all intersection legs to allow safer bike and pedestrian crossings. The largest change made to the intersection included condensing the intersection by bringing the south leg eastward, significantly shortening crossings. The intersection provides dedicated sidewalks and bike lanes, providing ample separation between active transportation and traffic.

The design meets client criteria by prioritizing active transportation, improving safety through traffic calming, and enhancing intersection intuitiveness. Raised concrete barriers at corners protect pedestrians and cyclists from right hooks. Reduced lane widths and curb radii help to slow cars down their turns and as they move through the intersection. Aligning intersection legs closer to 90 degrees provides a more typical intersection design, improving intersection familiarity for all users.

Further, the design meets projected transportation demands (peak hour volume of 950 vehicles in 20 years), with minimal vehicle delays to achieve a level of service (LOS) of A in the AM and B in the PM. Queue lengths formed were deemed acceptable, as they do not cause blockages at adjacent intersections and driveways.

The client has requested the design of an eight-metre-tall sign that establishes the intersection as a prominent gateway to campus. After consideration of precedent examples, Capstone Consulting has developed a mixed-material structure composed primarily of reinforced concrete, featuring pressure-treated timber and steel accents. Integration with existing campus architecture was prioritised, providing a sense of unity and significance to the area.

The proposed rainwater management system plans to mitigate cliff erosion issues by retaining rainwater onsite. After thorough consideration of an underground retention tank and implementation of rainwater reuse, it was decided that these systems would not be efficient or economically feasible. As such, our rainwater system will consist of green infrastructure. The main storage component is an infiltration swale with retention capabilities, recommended as the most sustainable and efficient solution.

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1.0 SITE INTRODUCTION

1.1 SITE OVERVIEW

The intersection of Chancellor Boulevard and East Mall connects to several key areas around the University of British Columbia (UBC) Point Grey campus. The project site is fronted by Regional Park to the north, Chancellor Place Neighborhood to the east, and the UBC campus to the south and to the west (Figure 1).



Figure 1: Project Site

1.2 SITE DESIGN CONSTRAINTS AND ISSUES

Several factors have influenced the development of the detailed design. One of the main challenges is the irregular geometry of the intersection. The skewed layout, due to the angles at which the approach legs meet, limits design flexibility. Additionally, the property lines at the Northeast and Southeast corners and the perimeter of Pacific Spirit Park are in close proximity to the intersection. This significantly reduces the available space for geometric improvements or the addition of infrastructure such as wider sidewalks, corner radii adjustments, or dedicated turning lanes. As a result, such constraints in combination made it infeasible to achieve a 90-degree intersection (known as normalization of the intersection). Normalization is typically preferred to improve driver sight lines and simplify navigation of the intersection.

Another key constraint is the risk of cliff erosion near the site. According to the UBC Stormwater Management Plan (SMP), infiltration-based stormwater systems are discouraged in this area due to the presence of weak slopes. As a result, all stormwater runoffs must be retained and managed on-site to prevent further destabilization of the slope. This constraint has influenced the preliminary water management approach and limits the types of green infrastructure that can be implemented.

Sightline visibility is also a concern at the northeast corner of the intersection, where tall bushes obstruct views for drivers and cyclists approaching from multiple directions. This vegetation limits the ability to see oncoming traffic and pedestrians, particularly when turning or crossing the intersection. Improving visibility at this corner will be important for enhancing safety and ensuring all users have adequate time to react.

Despite these limitations, the proposed design aims to create a safer and more user-friendly intersection by reallocating space and refining vehicle paths where possible, while ensuring that stormwater is managed responsibly in accordance with site-specific geotechnical constraints.

1.3 PROJECT AND DESIGN OBJECTIVES

The redesign of the Chancellor Boulevard and East Mall intersection is a key step towards improving the accessibility and safety of the UBC campus. The intersection, currently used as a minor entry point, incorporates highway-style design elements that challenge urban mobility. With its facilitation of high travel speeds, large and unintuitive layout, and lack of accommodation for active modes of transportation, the current East Mall intersection design fails to meet modern design standards and user expectations.

The primary objective of our project is to improve active transportation in the area, promote sustainability, and enhance safety for all road users. Capstone Consulting makes sustainability a key pillar and heavily emphasises user safety and comfort for all modes of transportation. The redesign aims to follow British Columbia Ministry of Transportation and Infrastructure (BC MoTI) standards to create a safe, accessible, and ecologically mindful gateway to the UBC campus.

1.4 TEAM CONTRIBUTIONS TO DETAILED DESIGN

The following table details each team member's contributions to the development of the detailed

design and the preparation of this report.

Table	1: Team	Member	Contributions
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Team Member	Task Type	Tasks
Matthew	Report	Intersection Analysis - Synchro, Gateway Design, Service Life Maintenance Plan
Buss	Design	Intersection Analysis - Synchro, Gateway Design, Structural Gateway Calculations, Gateway Details
Ting Foi	Report	Letter of Transmittal, Stakeholder Consultation Plan, Cross Sections and Grading, Erosion Settlement Control Plan
Ting rei	Design	CIVIL 3D Corridor, Notes, Grading, Sample Line Plan View, Profile View, Section Views
Sharar	Report	AutoTURN Analysis, Signage and Pavement Markings
Islam	Design	Signage and Paint Plan, 3D Renderings
Catherine Munro	Report	Executive Summary, Design Rationale, Design Overview, Design Constraints/Issues, Volume Projections, Service Life Maintenance Plan, Methodology, Active Transportation, Overall Report Review, Utility Coordination
	Design	Projected Volume Analysis, Standards Drawings, Service Maintenance Life Plan, Class A Cost Estimate
Peter	Report	Gateway Design, Construction Phasing and Traffic Management, Project Schedule, Risk Assessment
Tesson	Design	Project Management (Construction Schedule, Phasing, Project Charter, Traffic Management Plans), Gateway Design, Gateway Geotechnical Calculations
Adoloido	Report	Design Overview, Design Rationale, Rainwater Management Plan, Active Transportation, Utility Coordination
Zhang	Design	Final Intersection Design, Rainwater Management Design, Rainwater Calculations, Drawings (Key Plan, Signage and Pavement Markings, Rainwater Detail, Overall Drawing Review)

2.0 FINAL INTERSECTION DESIGN

2.1 DESIGN OVERVIEW

The final intersection redesign is a signalized protected intersection (see Figure 2). Geometrically, the intersection footprint has been significantly condensed by shifting the south leg eastward. This adjustment allowed us to implement pedestrian crosswalk at reduced lengths while ensuring no driver sightline issues. On the south side of the intersection, elevated bike lanes are separated from pedestrian sidewalks with flush concrete strips, while grass boulevards and concrete medians provide protection from vehicle traffic. On the north side, space and tie-in constraints require the use of multi-use pathways. Existing bike lanes integrate smoothly into the design through bike ramps. The gateway sign, located on the southwest intersection leg, features lightweight steel letters spelling "UBC," secured to a steel frame and mounted on a concrete column with welded steel plate connections. The rainwater management system includes a bioretention swale with daylighted storage in the southwest green space and a rain garden in the east leg median for added infiltration capacity.



Figure 2: 3D Model of Detailed Design

2.2 DESIGN RATIONALE

Aside from following design standards such as adhering to BC MoTI and emergency response requirements and minimizing tree loss, it is critical to ensure the design meets all client and stakeholder criteria. The table below provides justification for design elements and how design features address the client criteria. See Table 2 for a summary of key design components.

Client Criteria	Design Feature(s) that	Justification
	Address the Criteria	
Improve Intersection Intuitiveness for All Users	Condensed intersection	The intersection is less askew, more intuitive, and has improved sightlines for vehicle users.
Reduce Speeding Caused	Reduced curb radii at intersection corners	Lowers vehicle speeds, ensuring motorists navigate turning movements more cautiously.
by Highway-like Design	Narrowed road widths (3.5 m)	Reduces vehicle speeds and removes the highway-like existing conditions, increasing overall intersection safety.
	Reduced crosswalk lengths (less than 20m)	Allows for shorting crossing times, reducing the amount of time cars and pedestrians are interacting.
Enhance Pedestrian and Cyclist Safety and Prioritize Active Travel Modes	Dedicated bike lanes and crossings (2m)	High-visibility markings clearly delineate bike movements and spaces where cyclists have priority; reduces vehicle-cyclist interaction.
Tioues	Raised concrete medians	Provides cyclists protection from vehicle right-turn movements and minimizes vehicle-cyclists conflicts by providing dedicated bike lanes.
Create a Welcoming Gateway to UBC Campus	Gateway design placed in a highly visible area	Meets the 8-metre height requirement and effectively welcomes travellers to the UBC campus.
Retain all Rainwater on Site	Bioretention Swale, Raingarden, and Front Green Boulevards	Retains stormwater for 100-year storm event with additional capacity. Front green boulevards protect the bike lane from vehicle traffic, while capturing and treating stormwater.

Table 2: Rationale for Key Design Components

3.0 METHODOLOGY

3.1 ASSESSMENT OF CURRENT SITE CONDITIONS

Repeated site assessments were conducted throughout each project phase to understand existing conditions. The first site visit confirmed that the current intersection has a two-way stop on the Northbound and Southbound approaches. Immediate issues included: a median placed in the centre of the intersection, making it difficult for Southbound vehicles to travel straight through the intersection; poor sightlines on the North leg of the intersection creates safety concerns, especially for vehicles approaching at high speeds; lack of active transportation infrastructure; and no pedestrian crosswalks.

Following site assessment, we reviewed quantifiable data. Review of LiDAR data provided for this project showed a steep northward slope across the intersection, with a minimum right-of-way width of 33 m. ICBC Collision data recorded 12 vehicle collisions between 2019-2023, a relatively high rate compared to other local intersections. One pedestrian- vehicle collision was recorded at the intersection a little west of the crosswalk.

Existing traffic count data was used to predict future demands at the intersection (see Section 5.2.1) as well as provide an idea of traffic volumes. High 4-hour vehicle volumes along Chancellor Boulevard warranted to design for a signalized intersection. Geotechnical reports and borehole logs were used to assess foundation support and drainage conditions at the site. The investigation revealed that the subsurface at UBC has a significant layer of glacial till. Due to its low permeability, this layer presents drainage challenges at the project site.

3.2 REGULATORY PROVISIONS

Permits that are required to have the design approved and advanced to construction should be applied early to not cause delays in the schedule. Permits required for this project have been listed below (Table 3). Regulations and design guidelines will be monitored closely to ensure current design and construction standards are being met through the project. Table 3: Summary of Anticipated Permits Required

Permit/Review	Approval By:	Description
Development and Building Permits	UBC Campus + Community Planning	Required for significant intersection changes (covers environmental consideration, ensures the intersection aligns with campus' transportation goals)
Streets and Landscape Permit	UBC Campus + Community Planning	Required for any changes, excavation or construction on the land
Electrical Work Permits	Technical Safety BC	Required if new traffic signals or streetlights are being installed
Notice of Project	WorkSafe BC	Required for projects involving significant construction activity
Traffic Management Plan	UBC Campus + Community Planning	Required if construction will significantly disrupt traffic
Highway Use Permit	Ministry of Transportation and Infrastructure	Required for construction that affects access to any provincial roads near UBC (ex. University Boulevard, Marine Dr.)

3.3 STANDARDS AND SOFTWARE

Multiple standards and software were used to achieve the final design. Refer to Table 4 for a summary of the standards used and Table 5 for the software used.

Table 4: Design Standards Used

Design Standard	Purpose	
BC MoTI Design Standards	Compliance with design standards, design development and	
	guidance, and design drafting	
BC MoTI Traffic Management	Guidelines for traffic management and control in work zones	
Manual for Work on Roadways	ensuring protection of workers and the safe and efficient	
2020	movement of road users through the work zone.	
BC MoTI Cost Estimate	Used for Class A cost estimate for roadworks and utility	
Guidelines	construction	
Manual of Standard Traffic Signs	Provides standard sign designs, sizes, and placement	
and Pavement Markings	guidelines to ensure consistency, visibility, and	
	effectiveness of traffic control devices throughout the	
	roadway network.	

Design Standard	Purpose	
City of Vancouver Engineering	Compliance with local design standards, rainwater storage	
Design Manual 2019	capacity calculations, design development and guidance,	
	and design drafting	
City of Vancouver Standard	Drawing package, detailed design specifications, and	
Detailed Drawings 2019	grading design	
UBC Integrated Stormwater	Rainwater design development and guidance	
Management Plan		
BC Building Code 2018	Climatic and Seismic Data for gateway design	
CSA A23.3	Design of concrete structures	

Table 5: Design Software Used

Software	Purpose	
Autodesk AutoCAD	2D Engineering Drawings	
Autodesk Civil3D	Grading and Corridor Design	
SketchUp 2025	3D Intersection Model	
Autodesk Inventor	3D UBC Gateway Sign Model	
Microsoft Project	Gantt Chart Project Schedule	
Synchro	Intersection Analysis	
Lumion	3D Rendering	

3.4 INDIGENOUS CONSULTATION PLAN

The University of British Columbia (UBC) Vancouver-Point Grey campus and University Endowment Lands are located on the traditional, ancestral, and unceded territories of the x^wməθk^wəy' əm (Musqueam) people. As such, engaging with the Musqueam Indian Band was essential to ensure Indigenous perspectives and cultural sensitivities were respected in the detailed intersection design. We developed an Indigenous Consultation Plan specifically to involve the Musqueam community in this project. UBC's Indigenous Strategic Plan (ISP) served as the main guide for the consultation plan. As the framework of Indigenous engagement and reconciliation, the ISP outlines methods for fostering Indigenous inclusion through seven overarching goals and 37 actions. The methods applicable to Capstone Consulting's Indigenous stakeholder consultation plan can be found in Figure 3.

The ISP guided the incorporation of cultural protocols and Indigenous communication styles into our consultation. The foundational principles of the ISP ensures the process respects Indigenous values, acknowledges the impacts of colonialism, and aims to repair historical wrongs. This ensures that the Musqueam community will play an active role in the project as we consider Indigenous perspectives on environmental sustainability, traditional land stewardship, and cultural practices in the design processes. The ISP framework will also align the consultation with the United Nations Declaration of Rights of Indigenous Peoples (UNDRIP) and the Truth and Reconciliation Commission of Canada's Calls to Action.

Moreover, the ISP highlights the importance of building genuine, long-term relationships with Indigenous communities. As such, a consultation plan driven by the ISP will emphasize early and continuous engagement with Indigenous stakeholders. It will be vital to maintain ongoing communications throughout the project lifecycle, sharing progress and outcomes with Indigenous communities. We will take accountability to ensure that the Indigenous consultation process will lead to tangible actions and changes that reflect the feedback and needs of Indigenous stakeholders. We hope to foster a mutually beneficial relationship between University Endowment Lands and Indigenous communities that result in long-term partnerships that continue beyond the immediate project.

CIVL 446 2025W – DETAILED DESIGN FINAL REPORT CHANCELLOR BOULEVARD & EAST MALL INTERSECTION REDESIGN



Figure 3: Indigenous Stakeholder Consultation Plan

3.5 PUBLIC CONSULTATION PLAN

To ensure our design effectively addresses community needs, we will implement a comprehensive Public and Community Consultation Plan. Collaboration with UBC Campus and Community Planning (C+CP), which oversees campus development, will guide the establishment of a consultation timeline and ensure effective community outreach. The plan will progress through multiple phases to address various aspects of stakeholder engagement and project development. The first phase will focus on identifying issues and assessing community needs by gathering feedback on existing intersection challenges and other concerns. This input will be analyzed to highlight priorities and preferences, shaping the project's design objectives. A transportation plan advisory committee will review the findings, provide additional input, and ensure that any gaps in the engagement process are addressed. If necessary, the first phase will be repeated to refine the objectives.

The second phase will emphasize direct stakeholder engagement and the integration of feedback. Public open houses will present the project, detailing its potential impacts and benefits while allowing attendees to provide direct feedback. Online consultations, including surveys, forums, email updates, newsletters, and social media outreach, will broaden participation and ensure accessibility for all community members. The feedback collected during this phase will be summarized in newsletters and formal reports shared with stakeholders and integrated into the project's design and implementation plan.

Pre-construction coordination with stakeholders will occur in the third phase. Building on earlier engagements, this phase will focus on developing a construction timeline in collaboration with UBC C+CP and ensuring all affected parties receive timely communication. During the construction phase, the project team will work with UBC to disseminate relevant information, such as lane closures and detours, before and during each stage of construction. Communication efforts will specifically target nearby residents, intersection users, adjacent landowners, and building occupants, while also utilizing public channels like the project website to keep the broader community informed.

The final phase will focus on a post-construction education campaign. This campaign will ensure safe and efficient use of the intersection through a combination of road signage, community flyers, online resources, and staged construction that allows drivers to become familiar with the new infrastructure as it develops. Educational efforts will begin with construction and continue for at least four months after the roundabout becomes operational.

4.0 INTERSECTION DESIGN

4.1 GEOMETRIC DESIGN

Key features include a compacted intersection with narrowed travel lanes (3.5 meters typ.) that improve driver sightlines. Thie reduced lane widths reduce vehicle speeds, improve pedestrian visibility, and shorten crossing distances. Left-turn bays are introduced on all approaches except the westbound leg of NW Marine Drive, enhancing operations by separating turning movements from through-traffic lanes and reducing congestion.

Raised medians at intersection corners facilitate safe turning movements, and provide space for traffic control devices, lighting, and serves as a pedestrian waiting area before the crosswalk. Reduced curb radii further enhance safety by forcing drivers to slow down during turning movements while accommodating large vehicles such as I-BUS and WB-20 trucks. These elements together create an intersection that balances operational functionality with multi-modal accessibility. Figure 4 illustrates the geometric design of the intersection; Table 6 describes the geometric design characteristics.



Figure 4: Plan View of Intersection Geometric Design

Geometric Design Elements	NW Marine Drive (West Leg)	NW Marine Drive (North Leg)	Chancellor Blvd. (East Leg)	East Mall (South Leg)
Road Class	Primary Arterial	Primary Arterial	Primary Arterial	Collector
Posted Speed (km/hr)	50	50	50	30
No. of Approach Lanes	3	3	2	3
Dedicated Left Turn Lane	Yes	Yes	No	Yes
Vehicle Lane Width (m)	3.5 (EB Through) 3.5 (EB Left) 3.5 (WB Through)	3.8 (SB Through) 3.5 (SB Left) 3.5 (NB Through)	3.5 (WB) 3.5 (EB)	3.5 (NB Through) 3.5 (NB Left) 3.5 (SB Through)
Bicycle Lane Present	Yes (both directions)	Yes (SB)	Yes (both directions)	Yes (both directions)
Bicycle Lane Width (m)	2.0 with 1.5m boulevard (EB) 4.0 MUP with 1.5m buffer (WB)	N/A (NB) 3.0 MUP (SB)	2.0 with 1.5m boulevard (EB) 4.0 MUP with 1.5m buffer (WB)	2.0 with 1.5m boulevard (NB) 2.0 with 1.5m buffer (SB)
Median Present	Yes	Yes	Yes	Yes

Table 6: Geometric Design Elements

4.2 ACTIVE TRANSPORTATION

The proposed intersection design integrates a range of features aimed at enhancing safety, comfort, and accessibility for active transportation users. The design aligns with best practices and City of Vancouver standards to promote walking and cycling as viable, convenient, and safe modes of travel.

Pedestrian and cyclist crossings are fully signalized, providing dedicated and protected movement phases that eliminate conflicts with turning vehicles. To mitigate the risk of right-turn vehiclecyclist collisions, raised concrete islands at intersection corners physically separate cyclists and pedestrians from vehicles at high-conflict zones (see Figure 5). Reduced curb radii further increase safety by encouraging lower vehicle turning speeds. This gives drivers more time to react to cyclists and pedestrians crossing the intersection.



Figure 5: Raised Concrete Corner Barrier

Bike lanes, each 2 metres in width, are elevated to the adjacent sidewalk and separated from the pedestrians via a flush concrete strip. Raised bike lanes increase cyclist visibility and reinforcing separation from vehicle traffic. Bike ramps are provided at strategic locations to maintain smooth transitions between the existing cycling network and the proposed intersection infrastructure (Figure 6). In combination with green-painted bicycle crossings, these elements help guide cyclists intuitively and clearly through the conflict zones.



Figure 6: Bike Ramp Transition

Crosswalk lengths have been minimized to less than 20 m through geometric design refinements, reducing pedestrian exposure to traffic and shortening crossing times. Concrete barriers and rain garden boulevards provide additional horizontal separation between travel lanes and active transportation routes, enhancing user safety while contributing to stormwater management and streetscape quality.

Best practice design principles were applied throughout the intersection layout. All curb ramps are designed with letdowns and tactile warning strips to support accessibility for individuals with mobility and visual impairments. Audible pedestrian signals will follow City of Vancouver signal specifications, providing unique sound cues for each crossing direction to aid wayfinding. On the northeast corner, where private-owned vegetation currently impedes pedestrians clearly locating the crosswalk, a wayfinding tiles will be installed to guide visually impaired users to navigate the crossing safely (see Figure 7). Additionally, yield-to-pedestrian signage will warn cyclists.



Figure 7: Bike Lane Letdowns and Wayfinding Tiles

4.3 CROSS SECTIONS AND GRADING

The proposed road cross section follows the City of Vancouver design standards for local and residential streets and lanes, selected based on East Mall's classification as a collector road and its anticipated loading conditions. The pavement structure consists of a 35 mm asphalt concrete (AC) surface course over a 40 mm AC lower course, providing a durable and smooth riding surface.

Beneath the asphalt, a 150 mm granular base is included for structural support, placed over a 300 mm granular subbase to distribute loads and improve drainage (see Drawing 21). This configuration is capable of supporting the required structural capacity to accommodate regular vehicle traffic, including transit and service vehicles, while maintaining durability, ease of maintenance, and alignment with municipal design practices.

The slope design for the intersection follows the 2019 City of Vancouver Design Manual to ensure proper drainage and accessibility. Sidewalks throughout the project area are designed with a consistent 2% cross slope to promote surface runoff while maintaining pedestrian comfort and compliance with accessibility standards. For the roadway, the north, east, and west legs incorporate a 2% crown slope, directing runoff toward the curbs to minimize water pooling at the centerline. Due to the unique grading conditions along the southern leg, a 4% cross slope is applied to direct surface runoff toward the west side, toward the location of the bioretention swale. Where new construction ties into existing surfaces, a 0.5:1 cut slope and a 2:1 fill slope are used to ensure stable transitions and minimize soil displacement, aligning with both safety considerations and local design practices (see Drawings 8-11).

4.4 SIGNAGE AND PAVEMENT MARKINGS

The signage and pavement marking plan for the proposed intersection, detailed in Drawing 12, aligns with BC MoTI Traffic Signs and Pavement Markings Manual while addressing specific design needs. Key elements are summarized in Table 7.

Sign Code	Description	Placement/Guidelines
Yield to Bicycle Lane Users (R- 056-7e)	Indicates that vehicles must yield to two-way bicycle and multi-use traffic crossing the vehicle path. The green panel with bike and diamond symbols	 Typically installed where a right-turning vehicle crosses a bi-directional bike lane or multi-use path. Used in locations where drivers must yield before merging across or into an active transportation route.

Table 7: Sign Descriptions

Sign Code	Description	Placement/Guidelines
	designates a protected or dedicated active transportation corridor.	 Ensures clear priority for vulnerable road users and helps reduce collision risk at crossing points. In this drawing, it is used at key merge points—such as the Chancellor Blvd right- turn channel and near East Mall crossings— to reinforce yielding behavior for turning vehicles.
SHARED PATHWAY ENDS Shared Pathway Ends (B-R-102-Ta B-R-102-Tb)	Indicates the termination of a shared pathway where pedestrian and cyclist traffic must separate or yield.	 Located at multi-use pathway transitions (e.g., north-east and west of the NW Marine Drive and East Mall intersection). Used to clarify changes in active mode priority or separation.
Object Marker (W-54 R)	Marks obstructions on the roadway that are located on the motorist's right.	 Placed to align with the inside edge of the obstruction. Provides clear warning of objects that encroach on the traveled roadway, such as raised medians or barriers.
STOP LINE Stop Line Sign (RB-25)	Identifies the location where vehicles must stop at a stop- controlled intersection.	 Installed on East Mall facing northbound traffic. Used where the stop line may not be intuitive or easily visible to approaching drivers.

4.5 INTERSECTION ANALYSIS

4.5.1 PROJECTED VOLUME ANALYSIS

Turning movement counts from AM and PM peak hours were collected in early November. These traffic volume counts (see Appendix A) were projected to 2045, as stated in the Ministry of Transportation and Infrastructure's document, Planning and Designing Access to Developments, an intersection design should have a maximum design life of 20 years. These volumes were used in Synchro modelling to determine queue lengths and delay times (see Section 4.5.2).

To predict future traffic volumes, trends in population at the UBC was considered along the campus' mode share. UBC Campus and Community Planning has predicted that from 2021 to 2041, there would be a population growth of 13,000 students enrolled at UBC and an increase of 11,500 residents in the UBC neighbourhood (see Table 8). Assuming the population growth to increase linearly per year, this would average to 1225 people increase per year. This growth trend directly relates to the number of daily person trips to and from UBC. However, there are other factors such as the modal share on campus to determine the projected number of vehicle users. According to UBC's collected data, there has been a downward trend in the number of single-use vehicles on campus. The measured linear decrease is 400 people per year. Considering all these factors, it was determined there would be a 1.4% increase per year in traffic volumes.

Population Growth	2021	2041	Population Change Per Year
Residents in UBC Neighbourhoods	12500	24000	575
Student Enrollment	47294	60294	650
Mode Share	1997	2012	Population Change Per Year
Mode Share SOV	1997 45000	2012 39000	Population Change Per Year -400

Table 8: Population Growth Calculations

Total growth in vehicles = $575 + 650 - 400 = 825 \frac{veh}{year}$

Total % *growth in vehicles* =
$$\frac{825}{12500 + 47294} = 1.4\%/year$$

Total growth in active transportation =
$$\frac{13.33 \text{ cyclist/year}}{1600 \text{ cyclists}} = 0.83\%/\text{year}$$

To determine pedestrian and cyclist projected volumes, the mode share of cyclist and pedestrians was used as well as the population growth trends. Overall, there is a decrease in cyclist volumes, however, in more recent years there has been a steady increase. Therefore, only more recent years were used to determine the projected cyclist volumes. Overall, an increase of 0.83% per year was used to determine future cyclist and pedestrian volumes (see calculations above).

4.5.2 SYNCHRO ANALYSIS

Synchro traffic modeling software was utilized to analyze the performance of a signalized intersection with a focus on optimizing both vehicle flow and pedestrian safety. Through detailed simulations, Synchro provided estimates of maximum queue lengths for each approach under varying traffic volumes and signal timing scenarios. Additionally, the software's pedestrian modeling capabilities allowed for evaluation of pedestrian walking times, ensuring compliance with accessibility standards and enhancing safety. By balancing vehicle demand with adequate walk and clearance intervals, Synchro helped develop an optimized signal plan that reduced vehicle queues while improving pedestrian crossing opportunities.

Our design specifications meet the BC Supplement to the TAC Geometric Design Guide and the City of Vancouver Design Manual (CoV EDM 2019). The intersection was designed to accommodate a peak hour traffic flow of 950 vehicles per hour. The maximum vehicle delay in the intersection was determined to be 12.3 seconds per vehicle, inducing level of services of A and B for AM and PM peak hours, respectively. These levels of services are acceptable as they represent the most ideal traffic flow conditions for the intersection,

Signal phasing is optimized to ensure smooth vehicle and cyclist flow while maximizing pedestrian crossing time. Pedestrians are allocated 26 seconds to cross Chancellor Boulevard, providing ample time within the 35-second green phase. The signal operates under a semi-actuated, uncoordinated control system, keeping the northeast and southwest legs in the green phase until

a user is detected on East Mall, a minor road. However, green phase will be given to the minor road every three cycles to account for user and equipment errors.

Average cycle length was found to be 60 seconds, and 15% of cycles were found to have green time skipped for the minor roads. This meant that green time continued on the major roads until a user was detected on the minor road. Maximum cycle length was found to be 80 seconds, and minimum cycle length was 26 seconds. A signal phasing diagram is shown is Appendix B. Table 9 below summarizes cycle lengths.

Percentile	Cycle Time (s)	Green Time for Major Roads (s)	Green Time for Minor Roads (s)
10 th Percentile	35	35	0
50 th Percentile	53	39	7
Maximum	80	37	26

Table 9: Intersection Phase Signal Times

The maximum queue length in the SW leg (AM peak) towards the UBC campus is 92.6 meters and on the NE leg (PM peak) away from the UBC campus is 80.9 meters. (Refer to Table 10) These queue lengths are considered acceptable, as they do not obstruct adjacent intersections or driveways. The table below highlights the control delay per vehicle, as well as queue lengths.

Peak Hour	LOS	Projected Peak Hour Volume (Veh/Hr)	Control Delay (s / vehicle)	Maximum Queue Length (m)	Average Queue Length (m)
AM (8AM – 9AM)	Α	470	8.3	92.6 m	31.0 m
PM (4PM – 5PM)	В	950	12.3	80.9 m	38.3 m

Table 10: Intersection Level of Service and Queue Lengths

4.6 AUTOTURN ANALYSIS

As part of the detailed design for the Chancellor Boulevard & East Mall intersection, turning movement analysis was refined using AutoTURN and Vehicle Tracking software (see Appendix C). These tools modeled the full swept paths and overhangs for critical turning movements, validating the geometry against operational and safety requirements.

4.6.1 DESIGN VEHICLES

The detailed design incorporates the I-BUS and WB-20 (Figure 8) design vehicles. As stated in the BC Supplement to TAC Geometric Design Guide, "At a minimum, all turning movements should accommodate emergency vehicles; I-BUS, the TAC Inter-city bus is representative of such vehicles.



Figure 8: Design Vehicle Dimensions (TAC Geometric Design Guide for Canadian Roads, 2019)

These vehicles' turning radii were carefully considered to accommodate safe and efficient navigation through the intersection. For the I-BUS, a minimum outside front wheel turning radius of 19.8 m was chosen for vehicle speeds from 15–25 km/h (see Figure 9), while the WB-20 truck with higher speeds of 25–35 km/h required a minimum outside front wheel turning radius of 22.3 m.

DESIGN VEHICLE	I-BUS	WB-15 (BC)	WB-20	шт
SPEED	minimu	ım radius ⁽⁾ wh	^u of outsid eel	e front
0-15 km/h	15.2	13.7	14.5	13.6
15-25 km/h	19.8	17.7	17.7	17.7
25-35 km/h	19.8	22.3	22.3	22.3

Figure 9: Design Vehicle Minimum Radii

These design parameters directly affected the design of curb radii, lane widths, and the geometry of left-turn bays to allow larger vehicles to negotiate the intersection without encroaching into adjacent lanes or compromising safety in any other manner. By applying the TAC standards, the

intersection would support transit vehicles, emergency vehicles, and heavy trucks while sustaining the overall safety and efficiency of the layout for all road users.

4.6.2 TURN CONFLICT RESOLUTION

A major focus of the detailed design was addressing conflicts between eastbound (EB) and westbound (WB) right-turning vehicles and opposing through traffic, particularly for emergency vehicles requiring unobstructed movement.

To accommodate this:

- Rollover curbs were implemented on the south leg of the intersection;
- These curbs allow private vehicles to yield additional space by mounting the curb in situations where a large emergency vehicle needs turning clearance;
- This design strategy minimizes geometric expansion while maintaining operational flexibility; and
- The right-turn lanes were also widened and swept paths were validated to ensure that both
 I-BUS and WB-20 vehicles can turn without crossing into oncoming lanes or encroaching on pedestrian zones.

All turning paths were reviewed to ensure compliance with TAC design speed and envelope standards. This guarantees:

- Emergency vehicles can clear the intersection safely;
- Buses and freight vehicles can navigate without lane encroachment; and
- The design supports the long-term operational needs of both transit and goods movement while integrating active transportation.

4.7 UTILITY COORDINATION

Given the scope of the intersection redesign, including the complete lateral shift of the south intersection leg, significant utility coordination is required to ensure no service conflicts and minimize conflicts during construction. The following sections outline considerations and

proposed approaches for sewer tie-ins, street lighting, and street furnishings. See the Detailed Design Drawing Package for design specifications, proposed locations of new utilities, and construction information about utility coordination.

4.7.1 SEWER TIE-IN CONNECTIONS

The intersection realignment necessitates the relocation and reconstruction of storm sewer catch basins. It is the contractor's responsibility to locate and coordinate with the municipal utilities department to determine the exact location of existing underground utilities. This step will be essential to avoid damaging existing utilities during excavation and construction.

Existing catch basins within the construction area will be removed and disposed of. Where new catch basins are installed, catch basin leads will tie into existing storm system, as per CoV design standards. Geotechnical and topographic surveys will guide invert design and trenching to avoid utility clashes and ensure positive drainage through the intersection.

4.7.2 STREET LIGHTING

Street lighting will be upgraded and repositioned to align with the new intersection geometry and to meet CoV illumination standards for safety and visibility. Proposed lighting poles are located to match the shifted alignment and installed at pedestrian crossings, corners, and midblock locations as needed to ensure even light distribution (see Drawings 3-4).

Light selection was guided by CoV's lighting specs, with an emphasis on energy-efficient LED fixtures and smart controls where feasible. Pole placement has been coordinated with the location of other surface infrastructure, such as traffic signal poles, curb ramps, and bike lanes, to avoid conflicts.

4.7.3 STREET FURNISHINGS

Street furnishings will be updated and repositioned to enhance user experience and reflect the revised urban design of the intersection. Existing infrastructure within the project area will be removed and replaced with new street furnishings consistent with CoV streetscape guidelines.

New locations will be selected based on pedestrian desire lines and opportunities for passive surveillance. All furnishings will be located outside the pedestrian clear zone and will not obstruct sightlines or conflict with accessible infrastructure.

5.0 RAINWATER MANAGEMENT PLAN

5.1 DESIGN SPECIFICATIONS

The rainwater management plan was designed to manage a 100-year storm event, which requires 720 m³ of storage (see Section 5.2 for calculation methodology).

The recommended rainwater management strategy uses a retention and infiltration-based approach with green infrastructure across the site. The southwest green space will host the main retention system - a bioretention swale. The swale provides 1262 m³ of storage through surface ponding, bioretention soil, and a subsurface drain rock, exceeding our 720 m³ requirement. The east medium features a raingarden which provides an addition 174 m³ of storage. Key aspects of the bioretention swale are as follows:

- Surface and Subsurface Storage: A drain rock reservoir provides subsurface storage, with an impermeable fabric lining the perimeter to allow ponding. The maximum ponding level is 0.3 m, and safely drains away within 48 hours. Infiltration occurs only at the swale bottom. meeting CoV retention requirements.
- Overflow Management: A precast concrete lawn basin at the reservoir top safely redirects excess water, preventing surface backup in low-permeability native soils (<15 mm/hr).
- Water Quality Improvement: Water infiltrates through bioretention media, filtering contaminants and improving water quality.
- Curb Cut Inlet & Pretreatment: Covered curb cut inlets direct roadway runoff into the swale, while sediment basins reduce erosion and sediment load.

This infiltration-based design effectively manages stormwater on-site while avoiding the economic and logistical challenges of retention tanks. See Table 11 for design specifications.

Table 11: Rainwater Retention System Design Summary

Design Variable	Value
System Details	
Catchment Area	1.5 ha
Total Storage Capacity	1436 m ³
Infiltration Rate	3 mm/hr
Bioretention Swale	
Subsurface Storage Medium	Bioretention Soil: 55% course sand, 25% silty loam, 20% compost
Maximum Ponding Depth	0.3 m
Overflow Mechanism	Precast concrete lawn basin as per CoV STD DWG. S11.3
Sediment Control	Pretreatment sediment basin at the top of the reservoir as per CoV
Mechanism	STD DWG. GI12.6
Retention Mechanism	Impermeable liner with non-woven geotextile along trench walls
Raingarden	
Raingarden Vegetation	Cattails, Bulrush, Iris, Plantain, Common Reed, Sedge, Giant Mana
(Common Name)	Grass
Maximum Ponding Depth	0.15 m

5.2 STORAGE VOLUME CAPACITY

Due to UEL's lack of stormwater standards, calculations were performed with guidance from CoV regulation, the closest neighboring municipality. The required storage volume was calculated as per the City of Vancouver Building By-law (VBBL) dated July 23, 2019 and the City of Vancouver (CoV) Engineering Design Manual (EDM). Modified rational method was used to determine the required storage volume, represented by the area between the inflow and outflow hydrographs. The EDM informed runoff coefficients for different surfaces, which was used to develop composite runoff coefficients. Likewise, values from CoV's 2014 IDF curve for a 5-year return period and 2100 IDF curve for a 100-year return period were used for pre-development, and post development calculations, respectively. (See Appendix E for the modified rational method calculations).

The VBBL requires the maximum of the following conditions to be the required storage volume:

1. The peak flow rate discharged to the storm system under post-development conditions shall not be greater than the peak flow rate discharged under predevelopment conditions.

2. For green infrastructure, the first 48mm of rainwater in a 24-hour period from the site area shall be detained, and the detention volume requirement shall be calculated as the volume of water that would be present if water 48mm deep covered the entire site. This volume may be reduced by any combination of retention practices by subtracting rainwater capture potential, which is calculated as rainwater storage potential in growing medium multiplied by the medium volume, plus as applicable the storage volume within a subsurface reservoir layer and the volume infiltrated into the subgrade during a 24-hour period.

Condition two was the governing storage volume (720 m³, opposed to 282 m³ from condition one). The 720 cubic metres was then reduced by the retention methods outlined in Section 5.1 to 0 cubic metres, thus removing the requirement for a storage tank. Detailed calculations and relevant assumptions are outlined in Appendix E.

6.0 GATEWAY DESIGN

6.1 DESIGN SPECIFICATIONS

The client has requested the design and construction of an eight-metre-tall sign that establishes the intersection as a prominent gateway to campus. After consideration of various precedent examples, Capstone Consulting has developed a mixed-material structure composed primarily of reinforced concrete. Light-weight stainless steel letters typographically spell "UBC", like existing models found on Point Grey and Robson Square campuses (Figure 10). The letters are hollow and secured by a steel frame interior, fixed to the concrete column through steel plates, anchor bolts and welded connections.



Figure 10: Gateway Sign

The sign features pressure-treated timber accents for contrast. Placed atop its foundation, visitors or nearby students may use it as seating. The foundation itself will be embedded 1 m deep into the fill to ensure geotechnical stability.

6.2 STRUCTURAL CAPACITY

Structural calculations were done to check the capacity of the sign. While no live load is present on the sign, snow load and dead load were considered when doing the structural calculations, as well as the self-weight of the sign. The first step was determining the snow load that acts on the sign. The climatic data of the area was found using BCBC 2018 Appendix C, giving a snow load (S_s) of 1.9 kPa and a rain load (S_R) of 0.3 kPa. Using these values and the importance factors, the ULS was found to be 1.81 kPa, the value used as the snow load for the design. The self-weight of concrete was assumed to be 23.5 kN/m³, while the self-weight of the steel signage was assumed to be 78.5 kN/m³.

The letters are to be welded onto steel plates embedded on the concrete column. Steel welded connection calculations were performed to find the optimal weld lengths to hold the letters in place. A factor of safety of 2 was used in the calculation of the weld lengths. The weld lengths were found to be 75mm. Calculations can be found in Appendix F. The concrete column was designed to be 6m tall, holding both snow load as well as the three steel letters. The total factored load acting on the column was found to be 111.3 kN. Four vertical bars of 10M reinforcement were found as

the optimal reinforcement for this column, meeting the minimum reinforcement as stated in the building code. Calculations can be found in Appendix F.

Wind and seismic loading were also considered for the sign. The wind load on the freestanding sign structure was calculated using a wind pressure of 0.48 kPa, based off calculations shown in Appendix C and the British Columbia Building Code. With a projected face area of 9 m² (based on a 6 m height and 1.5 m width) and a drag coefficient of 1.3 for flat vertical surfaces, the resulting horizontal wind force was estimated at 5.62 kN. Seismic force acting on the sign was similarly found by using seismic information provided in the BCBC. A fundamental period of approximately 0.238 seconds was estimated based on the structure's height of 8 metres, and the corresponding interpolated spectral acceleration was found to be 0.851. With a total factored structural weight of 799.48 kN, and assuming an importance factor of 1.0 and a force modification factor of 1.5, the resulting seismic base shear is approximately 289.9 kN.

Table 12 below summarizes the loads that the gateway sign was designed to withstand.

Load Type	Load (kN)
Snow Load	14.7
Wind Load	9.4
Seismic Load	289.9
Self-Weight	571.6

Table 12: Gateway Sign Loading Summary

6.3 GEOTECHNICAL CAPACITY

Effective stress analysis was employed to verify the soil was capable of carrying the loads induced by the gateway design. Applying Meyerhof's Formula, an extension of Terzaghi's bearing capacity theory, an ultimate bearing capacity of 1759 kPa was determined. Compact silty sand was conservatively chosen, in accordance with the most recent geotechnical assessment of the area (Piteau Associates, 2002). The allowable bearing capacity was found to be 880 kPa, satisfying the 508 kPa stress applied by the structure.

Due to the generally limited soil information provided to Capstone Consulting, some values were assumed to be typical, namely, shear strength parameters and soil cohesion. All references were
appropriately cited in the geotechnical calculations, which can be found in Appendix G. The general contractor is strongly advised to confirm current soil conditions are in corroboration with data to ensure accuracy of the capacity check.

A factor of safety rating of 2 classifies this as a "low risk significant" structure. The absence of a groundwater table suggests that any settlement will more than likely occur immediately opposed to slow consolidation. This is beneficial to the construction process, as it reduces delays and early signs of possible failure can be detected. Saturation, however, remains contingent on the success of the rainwater management solution. Under saturated conditions, compact silty sand is at risk of reduced shear strength and effective stress.

7.0 PROJECT MANAGEMENT

7.1 CONSTRUCTION PHASING AND TRAFFIC MANAGEMENT

The BC MoTI 2020 Traffic Management Manual for Work on Roadways prioritizes safety, efficiency, and minimal disruption in work zones. To facilitate these standards, construction work will be completed in three primary phases (see Figure 11).





Figure 11: Construction Phase Plan

The scope of Phase 1 focuses on East Mall and its approach. Serving as the critical path activity for the entire intersection project, reconfiguring the road geometry involves extensive excavation work making it impractical to maintain partial access during construction. A full road closure will ensure the safety of the public– particularly vulnerable road users– while maximizing productivity as crews may work continuously without needing to reopen lanes during shifts. Pedestrians and cyclists will be diverted to Iona Drive, guided by a temporary modular walkway. There is also the option to utilize the existing adjacent walkway through Chan Centre; both routes have been highlighted in the traffic management plan (see Drawings 17-20). Considering the street's low traffic volumes, there will be minimal impact to regular traffic with several detours possible (i.e. West Mall, Theology Mall). Emergency response services will not be impacted, as UBC's dispatch stations are all located south of the project site.

Another notable aspect of this stage is the installation of rainwater management system, in the form of a bioretention swale as detailed in Section 5.0. These activities require large quantities of soil and subsurface materials to be transported to and from the site. As such, a designated staging area has been allocated to Iona Drive beside the site trailer, minimizing the possibility of a spillover where trucks may impede traffic.

Phases 2 and 3 focus on exterior and interior lane work, respectively. Work will be performed in roughly 50-meter-long segments to ensure minimal disruption to traffic. Phase 2 concentrates on overhauling the sidewalks and installing raised bike lanes. A traffic management plan has been provided for a typical midblock and intersection segment. These layouts feature ample signage from all directions ensuring road users are aware of the work activity areas, with clear direction to designated detours. Pedestrians are diverted to the opposing sidewalks as necessary. Two-way traffic is permitted in most cases except for specific instances when the road width is too small, as is the case with NW Marine Drive (north).

Lastly, the interior lanes will be regraded and surfaced according to specifications. Each section requires two lifts; each lift being a layer of subsurface material as prescribed in Section 4.3. Like Phase 1 and its reconfiguration of East Mall, Phase 3 involves heavy excavation. However, a full road closure is considered unfeasible, given the large traffic volumes associated with this arterial

route. As such, vehicle flow will be single lane alternating with work performed primarily in the evening. Traffic is directed by a minimum of two traffic control persons, with the use of Automated Flagger Assistance Devices (AFADs) as necessary. This structure follows a successful precedent BC MoTI paving operation along Chancellor Boulevard and Marine Drive, worked on during the summer of 2024.

7.2 CONSTRUCTION SCHEDULE

Pre-construction activities commence April 14, 2025– two weeks prior to groundbreaking of Phase 1. Final material procurement and site permitting will take place to ensure construction activities occur as scheduled, adhering to all regulatory requirements. Reconfiguring East Mall's road geometry is the critical path activity for this project, since obtaining approvals for additional street use permits rely on restoring traffic. UBC's academic calendar and its anticipated traffic volumes were taken into consideration. Final exams for Term 2 of UBC's Winter Session are conducted until April 27, with graduation ceremonies held in the adjacent Chan Centre until May 29, 2025. The full road closure may have a moderate impact on accessibility of these events. However, clear signage and direction to Rose Garden Parkade aim to minimize confusion, further addressed in Risk Assessment (see Section 7.3).

Phases 2 and 3 occur during UBC's summer session, which also sees increased visitation to nearby parks like Wreck Beach. Due to the nature of paving work, traffic may need to be reduced to single-lane alternating thus necessitating nightwork. Project closeout (i.e. quality control, finishes) and demobilization will take three weeks with project completion on August 13, 2025, totalling a duration of 100 days. See Figure 13 for a summary of the schedule, and Appendix E for a complete construction schedule.



Figure 12: Construction Schedule Summary

7.3 RISK ASSESSMENT

A comprehensive risk assessment was conducted to identify potential issues that may arise throughout the lifecycle of the project. Capstone Consulting, alongside the elected general contractor, will continue to monitor these risks, implementing contingency plans where possible. Refer to Appendix I for the project charter, containing a complete risk matrix and grading schematic.

Currently, significant risks hinge on regulatory compliance and stakeholder concerns. Complexities of the project site arise from its irregular geometry. Although a thorough simulation of vehicle swept paths has been conducted (see Section 4.5.2) it remains probable that a spatial condition is unfulfilled. In such an outcome, the consequences would be dire– hence a 'Critical' status has been assigned for the general contractor to closely examine throughout the construction process. Furthermore, NW Marine Drive and Chancellor Boulevard are fronted by private residential property. The likelihood of stakeholder conflict contributes to a 'High' level of risk. A full list of risks and their respective mitigation approach is found in the project charter.

7.4 EROSION SEDIMENT CONTROL PLAN

To mitigate environmental impacts during construction and ensure compliance with relevant regulations, an Erosion and Sediment Control (ESC) Plan has been developed for the Chancellor

Boulevard and East Mall intersection. The ESC Plan integrates structural and procedural best management practices (BMPs) to reduce soil disturbance, manage runoff, and prevent sediment-laden flows from reaching nearby waterways. The use of silt fences, sediment traps, storm drain inlet protection, and a stabilized construction entrance will serve as the first line of defence in containing on-site sediment. Additional structural controls such as check dams, diversion ditches, and berms will be placed strategically across the site to manage the direction and velocity of runoff, particularly in areas with steep slopes. Stockpiled materials will be covered and enclosed with barriers, while catch basins will be outfitted with sumps and sediment traps to protect the storm system during rainfall events.

To support these physical measures, procedural BMPs will be implemented throughout construction to guide how and when land-disturbing activities occur. Land clearing and grading will be phased and limited to only active work zones, reducing the exposure of bare soil to rainfall and wind. Disturbed soils will be stabilized as soon as possible using hydroseeding, erosion control blankets, mulch, or sod, depending on site conditions. Construction will be scheduled to avoid periods of intense rainfall, and all ESC measures will be monitored and maintained regularly to ensure continued performance. Inspections will be conducted after major rainfall events to assess the integrity of installed controls, with repairs made as needed. These practices are consistent with requirements from the City of Vancouver and UBC Campus and Community Planning.

Following completion of construction, long-term erosion and sediment control measures will be implemented across the project site. Permanent BMPs will include vegetated landscaping, ripraplined ditches to control concentrated flow paths, and bioswales to filter stormwater and promote infiltration. Catch basins will also feature oil/grit separators to treat runoff before discharge. The rainwater management system and such measures align with UBC's Integrated Stormwater Management Plan and ensure that the redesigned intersection supports long-term environmental and ecological sustainability.

8.0 CLASS A COST ESTIMATE

The total design cost for the project, including 12% contingency, is \$6,416,040 (see Table 13). See Appendix J for a detailed breakdown of the cost estimate. Intersection design costs account for full excavation of the intersection, installation of new traffic signals, and repaving following design standards. The cost estimate of labour aligns with the 2023 ASEC BC Consulting Engineers Fee Guideline.

Table 13: Initial Construction Class A Cost Estimate

Task	Total (\$)
Construction Works - Intersection	\$2,844,370
Construction Works – Gateway Sign	\$23,960
Construction Works – Retention Tank	\$710,300
Project Administration	\$19,040
Labour and Equipment	\$2,130,940
Total (12% Contingency)	\$6,416,040

9.0 SERVICE LIFE MAINTENANCE PLAN

The service life maintenance plan includes activities related to the surface, drainage, traffic signals, vegetation, snow, gateway sign, rainwater swale and raingardens. Intersection maintenance standards are outlined in the BC MoTI design guidelines. The total cost of maintenance is estimated at \$1.3 million throughout the 20-year design life. Table 14 outlines the various costs associated with these activities. See Appendix K for a detailed table with all service life maintenance activities, frequency the task requires, and costs.

Tahle	14 ·Ser	vice I ife	Maintenance	Activities	and Costs
Table	14.001	VICE LITE	riannenance	Activities	

Task	Total Cost Over 20-Year Design Life
Surface Maintenance Activities	\$480,500
Drainage Maintenance Activities	\$17,200
Traffic Signal Maintenance Activities	\$170,000
Vegetation Maintenance Activities	\$192,000
Winter Maintenance Activities	\$26,000
Gateway Sign Maintenance Activities	\$25,200
Rainwater Swale and Raingarden Maintenance Activities	\$284,500
Total (12% Contingency)	\$1,339,000

Ongoing care and maintenance of the intersection must consider several environmental and operational factors throughout the design life. One key concern is the potential for contamination of downstream water bodies, as debris and pollutants from the roadway can degrade aquatic ecosystems. Road salt used during the winter to prevent slippery conditions can also contribute to runoff contamination, promoting issues like algae growth if not properly managed. Regular sweeping and cleanup of road surfaces is important, especially after snow events or heavy use, to minimize pollutant build-up and ensure safe driving conditions. When cleaning up aggregate, it is best done when surfaces are moist to prevent dust from being swept into the air and negatively impacting local air quality.

Drainage maintenance will also be an essential part of the long-term upkeep. This includes regular clearing of catch basins, addressing blockages in storm pipes, and completing storm sewer upgrades when necessary to ensure efficient water flow. Sediment should be removed from the pre-treatment sediment basin once 25–50% of its volume is filled, to maintain system performance.

In terms of physical infrastructure, curb maintenance on the northwest corner will be especially important. This is where a service vehicle regularly accesses the outhouse near the forested area, often by driving over the curb (see Figure 13). Vegetative maintenance should also be prioritized, with special attention to the tall hedges on the northeast corner of the intersection, which obstruct sightlines and pose safety concerns for drivers and cyclists.



Figure 13: Google Maps Streetview of Northwest Corner

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APPENDIX A

VOLUME PROJECTIONS

Turning Movement Counts Collected - Raw Data



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			←	t	↦	111			
			24	3	45	7			
			out↓	32	71	in 1			

APPENDIX B

SYNCHRO ANALYSIS

AM Peak Hour Analysis

SimTraffic Performance Report Baseline

3: Int Performance by approach

Approach	NB	SB	NE	SW	All
Total Delay (hr)	0.1	0.1	0.4	1.0	1.6
Delay / Veh (s)	9.7	7.4	5.0	7.7	7.0
Stop Delay (hr)	0.1	0.1	0.2	0.7	1.2
St Del/Veh (s)	9.5	7.7	3.4	5.3	5.1
Total Stops	47	36	62	185	330
Stop/Veh	0.85	0.84	0.23	0.38	0.39
Vehicles Entered	55	43	264	490	852
Vehicles Exited	55	43	264	488	850
Hourly Exit Rate	55	43	264	488	850

Total Network Performance

Total Delay (hr)	1.9	
Delay / Veh (s)	8.3	
Stop Delay (hr)	1.2	
St Del/Veh (s)	5.3	
Total Stops	330	
Stop/Veh	0.39	
Vehicles Entered	852	
Vehicles Exited	847	
Hourly Exit Rate	847	

Queuing and Blocking Report Baseline

2025-04-07

Intersection: 3: Int

Movement	NB	SB	SB	NE	NE	SW
Directions Served	L	L	TR	L	TR	LTR
Maximum Queue (m)	7.3	0.4	9.4	6.6	44.7	92.6
Average Queue (m)	0.5	0.1	0.6	0.7	9.6	31.0
95th Queue (m)	3.2	0.3	3.5	3.3	26.5	64.9
Link Distance (m)			39.8		132.2	88.0
Upstream Blk Time (%)						0
Queuing Penalty (veh)						0
Storage Bay Dist (m)	15.0	15.0		15.0		
Storage Blk Time (%)			0		2	
Queuing Penalty (veh)			0		0	

Nework Summary

Network wide Queuing Penalty: 0

Actuated Signals, Observed Splits Baseline

2025-03-06

Intersection: 3: Int

Phase	2	4	6	7	8
Movement(s) Served	NBTL	NET	SBTL	NEL	SWTL
Maximum Green (s)	35.0	37.0	35.0	4.0	29.0
Minimum Green (s)	4.0	4.0	4.0	4.0	4.0
Recall	None	Max	None	None	Max
Avg. Green (s)	11.1	60.8	11.1	4.0	50.2
g/C Ratio	0.14	0.76	0.14	0.01	0.73
Cycles Skipped (%)	18	19	18	80	6
Cycles @ Minimum (%)	7	0	7	20	0
Cycles Maxed Out (%)	0	81	0	20	94
Cycles with Peds (%)	11	11	7	0	20
Controller Summary					

Average Cycle Length (s): 64.9 Number of Complete Cycles : 54

PM Peak Hour Analysis

SimTraffic Performance Report

Baseline

3: Int Performance by approach

Approach	NB	SB	NE	SW	All
Total Delay (hr)	0.3	0.2	1.6	1.2	3.3
Delay / Veh (s)	11.9	11.2	9.2	13.3	10.7
Stop Delay (hr)	0.3	0.2	1.0	1.0	2.5
St Del/Veh (s)	12.1	11.2	5.7	10.9	8.0
Total Stops	69	44	244	156	513
Stop/Veh	0.71	0.76	0.38	0.50	0.46
Vehicles Entered	96	57	642	316	1111
Vehicles Exited	97	58	643	315	1113
Hourly Exit Rate	97	58	643	315	1113

Total Network Performance

Total Delay (hr)	3.8	
Delay / Veh (s)	12.3	
Stop Delay (hr)	2.5	
St Del/Veh (s)	8.2	
Total Stops	513	
Stop/Veh	0.46	
Vehicles Entered	1111	
Vehicles Exited	1115	
Hourly Exit Rate	1115	

2025-03-06

Intersection: 3: Int

Movement	NB	NB	SB	SB	NE	NE	SW
Directions Served	L	TR	L	TR	L	TR	LTR
Maximum Queue (m)	12.6	8.1	1.2	8.6	22.6	80.9	79.3
Average Queue (m)	0.5	0.6	0.1	0.3	1.1	38.3	24.9
95th Queue (m)	4.2	3.2	0.6	2.9	7.8	72.1	52.3
Link Distance (m)		63.5		39.8		132.2	88.0
Upstream Blk Time (%)							
Queuing Penalty (veh)							
Storage Bay Dist (m)	15.0		15.0		15.0		
Storage Blk Time (%)	0					15	
Queuing Penalty (veh)	0					2	

Nework Summary

Network wide Queuing Penalty: 2

Actuated Signals, Observed Splits

	_		
Baseline			

Intersection: 3: Int

Phase	2	4	6	7	8
Movement(s) Served	NBTL	NET	SBTL	NEL	SWTL
Maximum Green (s)	37.0	35.0	37.0	4.0	27.0
Minimum Green (s)	4.0	4.0	4.0	4.0	4.0
Recall	None	Max	None	None	Max
Avg. Green (s)	15.9	43.4	15.9	4.0	36.2
g/C Ratio	0.24	0.64	0.24	0.01	0.61
Cycles Skipped (%)	11	12	11	78	0
Cycles @ Minimum (%)	2	0	2	22	0
Cycles Maxed Out (%)	0	88	0	22	100
Cycles with Peds (%)	25	13	15	0	20
Controller Summary					
Average Cycle Length (s): 59.6				

Number of Complete Cycles : 60

Signal Phasing Diagrams



Ave Cycle



Max Cycle



APPENDIX C

AUTOTURN/VEHICLE TRACKING



WB & EB Left Turn Movements (Emergency Vehicle)



WB & EB Right Turn Movement (Emergency Vehicle)

APPENDIX D

3D RENDERINGS







Linnion



The second

50

















APPENDIX E

RAINWATER VOLUME TANK CALCULATIONS

Pre Development Flow - Proposed Areas

Onsite Areas and Runoff Coefficient

	Road*	Sidewalk**	Green Space***
A (Areas m ²)	4785	499	2728
Site Coverage (%)	60%	6%	34%
R (Runoff Coeffcient)	0.88	0.80	0.23
Site Coverage * R	0.53	0.05	0.08
Site Area A (m ²) =	8012	Composite R =	0.65

Post Development Flow - Proposed Areas

Onsite Areas and Runoff Coefficient

	Road*	Sidewalk**	Green Space***
A (Areas m ²)	3947	1274	2791
Site Coverage (%)	49%	16%	35%
R (Runoff Coeffcient)	0.88	0.80	0.23
Site Coverage * R	0.43	0.13	0.08
Site Area A (m ²) =	8012	Composite R =	0.64

*Roads are considered "Concrete Streets" per CoV Table 5-2

**Sidewalks are considered "Drives and Walks" per CoV Table 5-2

***Green Space assumed to be "Average (2-7%) Lawns, Heavy Soil" per CoV Table 5-2

100 Year Peak Flow Calculations

Q = RAIN Catchment Area: 1.5 Ha

1) Storage Volume Requirements As Per City of Vancouver for Green Infrastructure

Required Storage, as per COV:	48 mm
Required Duration:	24 hours

Storage Demand:	720.0 m ³

Because the storage volume computed via analysis of pre- and post development flows is 282.3 m³, the governing storage demand is the requirement for the storage of 48mm of rainwater in 24 hours, as per the City of Vancouver Building By-law dated July 23, 2019.

2) Rainwater Volume Absorbed by Native Soil Via Infiltration

By assuming an average infiltration rate of **3 mm/hr** from similar site conditions:

```
Native Soil Storage Capacity: 93.5 m<sup>3</sup>
```

3) Rainwater Volume Absorbed by Storage Medium

By assuming a storage medium porosity of **0.42** from the Hydrogeological and Geotechnical Assessment report of the UBC campus by Piteau Associates, and retrieving the volume of bioretention soil, drain rock, and sand medium from Civl3D:

Storage Medium Volume: 1855.2 m³

Storage Medium Storage Capacity: 779.2 m³

4) Rainwater Volume Stored Via Ponding

With the volume of water ponding from Civl3D, assuming a maximum ponding depth of **0.3m**,

Ponding Storage Capacity:	389.6 m ³
Total Infiltration Swale Storage Capacity:	1262.3 m ³ > 720.0 m ³

Note: The bioretention swale alone manages the 100 year storm event. The raingarden provides additional capacity.

Raingarden Storage Capacity Chancellor Boulevard and East Mall Intersection

100 Year Peak Flow Calculations

Q = RAIN Catchment Area: 1.5 Ha

2) Rainwater Volume Absorbed by Native Soil Via Infiltration

By assuming an average infiltration rate of **3 mm/hr** from similar site conditions:

Native Soil Storage Capacity:	30.4 m ³
-------------------------------	---------------------

3) Rainwater Volume Absorbed by Storage Medium

By assuming a storage medium porosity of **0.42** from the Hydrogeological and Geotechnical Assessment report of the UBC campus by Piteau Associates, and retrieving the volume of bioretention soil, drain rock, and sand medium from Civl3D:

Growing Medium Depth 0.45 m

Storage Medium Storage Capacity: 79.9 m°	79.9 m ³	Storage Medium Storage Capacity:
--	---------------------	----------------------------------

4) Rainwater Volume Stored Via Ponding

With the volume of water ponding from Civl3D, assuming a maximum ponding depth of **0.15m**,

Ponding Storage Capacity:	63.4 m ³	
Additional Storage Capacity:	173.7	
Detention Tank Storage Requirement Chancellor Boulevard and East Mall Intersection

CIVL446 07-Apr-25

Q= RAIN

100 Year Peak Flow Calculations

Site Area (Ha)= 1.5

	Тс	Runoff coefficient	Area	Intensity	n	Q
	min	%	На	mm/hr		cms
Qpre (2014 IDF)	10	0.65	1.500	93.9	0.00278	0.256
Qpost (2100 IDF)	5	0.64	1.500	191.3	0.00278	0.511

Storage Volume Required (CoV Rational Method)*

*Volume based on maintaining pre-construction runoff flows

Storage Volume =

Tank Storage Required =

$$V = \boldsymbol{Q}_p \boldsymbol{D} - \boldsymbol{Q}_0(\frac{\boldsymbol{D} + \boldsymbol{T}_c}{2})$$

D= Duration of storm, in seconds

 Q_p = Peak runoff rate for duration D, in cubic meters per second

Q0 = Maximum allowable discharge, in cubic meters per second

 T_c = Time of concentration, in seconds

Rainfall Duration D	Peak Runoff Q _p	Maximum Allowable Discharge Q₀	Time of Concentration T _c	Groundwater Inflow Q _{GI} * *Added to Peak Runoff	Required Storage
min	cms	cms	min	cms	cu.m
5	0.561	0.256	5	0.0500	106.58
15	0.324	0.256	5	0.0500	182.95
16	0.314	0.256	5	0.0500	188.24
17	0.305	0.256	5	0.0500	193.29
20	0.283	0.256	5	0.0500	207.18
30	0.235	0.256	5	0.0500	243.82
40	0.207	0.256	5	0.0500	271.05
45	0.197	0.256	5	0.0500	282.31

282.31 cu.m

APPENDIX F

GATEWAY - STRUCTURAL CALCULATIONS

Appendix F - Gateway Structural Calculations

Load Calculations:

Snow Load:

$$ULS = 1.0 * [1.9 * (0.8 * 1 * 1 * 1) + 0.3] = 1.82 kPa$$

Snow Load on Letters

$$Area = 0.4 m * 0.2m = 0.8m^{2}$$

1.82 kPa * 0.8m² = 1.456 kN = 1456 N

Snow Load on Column

$$Area = 0.9 m * 1.4 m = 1.26m^2$$

1.82 kPa * 1.26 $m^2 = 2.293 kN = 2293N$

Snow Load on Footing

$$Area = 3 m * 2 m = 6m^{2}$$

1.82 kPa * 6 m² = 10.92 kN = 10920N

Self Weight of Concrete Column

Volume =
$$6m * 1.4m * 0.9m = 7.56m^3$$

7.56 $m^3 * 23.5 kN/m^3 = 177.76 kN$

Self Weight of Steel Letters

Assuming hollow letters, with 0.05m thickness

Volume =
$$2m * 2m * 0.4m = 1.6m^3$$

Hollow Volume = $(2 - 0.05 * 2) * (2 - 0.05 * 2) * (0.4 - 2 * 0.05) = 1.083m^3$

Steel Volume =
$$(1.6 m^3 - 1.083m^3) * 0.67 = 0.347m^3$$

$$0.347m^3 * 7850 kg/m^3 * 9.81 m/s^2 = 26,650 N$$

26,650 N * 3 Letters = 79950 N = 79.95 kN

Steel Connection Calculations

Snow Load on Letter : 1456 N

Self Weight of Letter: 26,650 N

Load Combinations:

1.4 D = 37.31 kN <- Governs

1.25 D + 1.5 S = 35.49 kN

Weld Calculations:

Assuming fusion face to be 5mm.

Weld Metal Check:

$$Vr = 0.67 \phi_w A_w X_w (1.0 + 0.5 sin^{1.5} \theta) M_w$$

37310 N = 0.67 * 0.67 * 0.707 * Length of Weld * 1 * 490 MPa

 $L = 23.99 \, mm$

Base Metal Check:

$$Vr = 0.67 \Phi_w A_m F_u$$

37310N = 0.67 * 0.67 * 5 * 450 * Length of Weld

 $L = 36.94 \, mm$

Using a Factor of Safety of 2.0, use 74 mm Weld Length.

Concrete Column Calculations:

Concrete Dimensions: 6m * 1.4 m * 0.9 m

 $Ag = 1260000 mm^2$

Using 30 MPa concrete = $\alpha = 0.81$, $\beta = 0.90$

Snow Load = 3749 N

Dead Load = 79950 N

Load Combinations:

1.4 D = 111.3 kN <- Governs

1.25D + 1.5 S = 105.56 kN

Reinforcement Calculations:

$$P_{ro} = 0.8(\alpha \phi_c f'_c A_c + \phi_c F_y A_s)$$
111300 = 0.8 * (0.81 * 0.65 * 30 * A_c + 0.85 * 400 * A_s)
111300 = 12.636 A_c + 272 A_s
111300 = 12.636 * 0.99 * Ag + 272 * 0.01 * Ag
111300 = 12.18 Ag
 $Ag = 9133.8 mm^2 < - OK$
 $As = 0.01 * Ag = 91.34 mm^2$
Use 4-10M Bar (Nominal Area = 400mm²)

Clear Cover = 75mm

Tie Spacing = 480 mm

Concrete Footing Calculations:

Footing Dimensions: 3m * 2m * 2m

Footing Depth: 1m

Snow Load = 10920 N + 1456 N + 2293 N = 14,670 N

Dead Load = 111300 N + 177760 N + 282000 N = 571,060 N

Load Combinations:

1.4 D = 799.484 kN <- Governs

1.25 D + 1.5 S = 735.83 kN

Assuming 20M Bars

$$d_{Average} = 1000 - 75 - 1.5(20) = 895mm$$
$$dv = 0.9 * d = 805.5 mm$$
$$Pf/Af = 799.484 \ kN/(3m * 2m) = 133.25 \ kPa$$
$$C + Davg = 900 + 895 = 1795 \ mm$$
$$C + Davg = 1400 + 895 = 2295 \ mm$$
$$Vf = 133.25[(3 * 2) - (1.795 * 2.295)] = 250.92 \ kN$$

One Way Shear Check:

$$Vr = \phi \beta \sqrt{f'c} b_w d_v$$

$$Vr = 0.65 * \frac{230}{1000+805.5} * \sqrt{30} * 805.5 * 3000 = 1095.95 \, kN$$

Two Way Shear Check:

$$Vr = (1 + 2/\beta_c) * 0.19 * \phi_c \sqrt{f'_c} \le 0.38 \phi_c \sqrt{f'_c}$$

$$Vr = (1 + 2/(1.5)) * 0.19 * 0.65 * \sqrt{30} * 3000 * 805.5 = 3814.09 kN$$

Flexural Design

$$As = 0.002 * (1000 * 3000) = 6000 mm^{2}$$

$$6000/300 = 20 - 20M Bars$$

$$\beta_{1}c = \frac{\Phi_{s}F_{y}A_{s}}{\alpha_{1}\Phi_{c}f_{c}b} = \frac{0.85*400*6000}{0.81*0.65*30*3000} = 43.05mm < 0.5d$$

$$Mr = 340 * 6000 * (895 - 43.05/2) = 1781.9 kN * m$$

$$Mf = 133.25 kPa * 0.8 * 3 * 0.8/2 = 127.92 kN * m$$

$$Mf < Mr$$

APPENDIX G

GATEWAY - GEOTECHNICAL CALCULATIONS

Appendix G - Bearing Capacity Calculations

 $Mass = 52000 \ kg \ weight = 508 \ kN$ Dimensions = $\lim_{b \to ch} \times \lim_{width} = 6m^2$

Using Effective Stress Analysis

Shape Factor: $S_c = 1 + \frac{B}{r} \cdot \frac{N_q}{r}$

 $qult = C \cdot N_c \cdot S_c \cdot i_c \cdot d_c + \sigma_D \cdot N_q \cdot S_q \cdot i_q \cdot d_q + 0.5 \cdot \gamma \cdot \beta \cdot N_\gamma \cdot S_\gamma \cdot i_\gamma \cdot d_\gamma$

 $\phi = 27^{\circ} \quad \therefore \beta \cdot 27^{\circ}$

$$= 1 + \frac{2}{3} \cdot 13.2/23.94$$
$$= 1.4$$
$$S_q = 1 + \frac{B}{L} \cdot \tan \emptyset$$
$$= 1 + \frac{2}{3} \cdot \tan(27^\circ)$$
$$S_{\gamma} = 1 - 0 \cdot 4B/L$$

= 0.73

 $\beta = 0^{\circ}$,

 $\begin{array}{ll} \begin{array}{c} \cdot \gamma \cdot \beta \cdot N_{\gamma} \cdot S_{\gamma} \cdot i_{\gamma} \cdot d_{\gamma} & \hline d = \text{Depth factors} \end{array} \end{array}$ Depth Factor: $D_{F} = 1m, \ B = 2m \therefore D_{F}/B \leq 1$ $\therefore d_{c} = 1 + 0 \cdot 4 \times D_{F}/\beta$ = 1.8 $d_{q} = 1 + 2 \tan (1 - \sin \phi)^{2} D_{F}/B$ $= 1 + 2 \cdot \tan(27^{\circ}) \times (1 - \sin(27))^{2} \times \gamma_{2}$ = 1.2

= Shape factors

 $d_{\gamma} = 1$

= Inclination factors

Inclination Factor:

Angle of load ----- vertical

$$\dot{\cdot}_{i_{\gamma}} = \left(1 - \frac{\beta}{\phi}\right)^2 = 0$$

$$i_c = i_q = \left(1 - \frac{\beta^*}{90^*}\right)^2$$

$$= 1 - 0 = 1$$

$N_q, N_c, N_{\gamma} =$ Bearing Capacity Factors.

Will place the statue in fill; "compact sand to silly sand" conservatively, take compacted silty sand as main soil. No data given on strength;	∴, from Terzaghi & Peck, 1948: friction angle 270°: Meyerhof:
: Typical values. From: Carter, M and Bentley, S. (1991). Correlation of Soil Properties. PENTECH PRESS Publishers: London	$N_c = 23.94$
Loose silty sand: 27° friction angle. Note: Firm silt also has 27° friction angle. No groundwater table.	$N_q = 13.2$
σ_D' = Vertical Effective Stress @ Foundation Base level	$N_{\gamma} = 9.46$
$\sigma_D'=\sigma_D-u, u=0 since \ no \ GWT$ & Compact sand & silty sand has good drainage / permeability	
Unit weight	
$\sigma_D = \gamma \cdot Z, depth$	

Assume γ silty sand = 18 kN/m³

 $\therefore \sigma_D = 18kN/m^3 \times 1m$

 $18kN/_{m}^{2} = 18kPa$

 $\gamma' = 18kN/m^3$

Use C cohesion = 22kPa for silty sand

From "Swiss Standard SN 670 010b, Characteristic Coefficient of Soils, Association of Swiss Road and Traffic Engineers"

 $\therefore \, qult = 22 \cdot 23.94 \cdot 1.4 \cdot 1.8 + 18 \times 13.2 \cdot 1.3 \cdot 1.2 + 0 \cdot 5 \cdot 18.2 \cdot 9.46 \cdot 0.73 \cdot 1$

= 1327 + 370 + 62

 $= 1759 \ kPa$

FOS = 2.0 is not a very risk-significant structure

 \therefore qall = qult/FOS

 $qall = 879.5 \rightarrow Applied \ load = 508kN$

:-, It is acceptable to construct using as 3m imes 2m area embedding 1m depth into the fill

Note: During excavation, contractor should ensure site conditions reflect majority compact sand & silly sand

Using Sonic Hole TH01-01

APPENDIX H

CONSTRUCTION SCHEDULE

D	Task Name	Duration	Start	Finish	Apr
1	Intersection Redesign	100 days	Mon 4/14/25	Wed 8/13/25	
2	Pre-Construction	16 days	Mon 4/14/25	Mon 5/5/25	
3	Procurement	11 days	Mon 4/14/25	Mon 4/28/25	
4	Permitting	11 days	Tue 4/15/25	Tue 4/29/25	
5	Site Mobilization	5 days	Tue 4/29/25	Mon 5/5/25	-
6	Site Trailer	1 day	Tue 4/29/25	Tue 4/29/25	-
7	Machinery	4 days	Wed 4/30/25	Mon 5/5/25	
8	Phase 1 - East Mall	26 days	Thu 5/1/25	Thu 5/29/25	
9	Remove Existing Works	8 days	Thu 5/1/25	Sat 5/10/25	
10	Clearing and Grubbing	2 days	Thu 5/1/25	Fri 5/2/25	
11	Demolition	3 days	Mon 5/5/25	Wed 5/7/25	
12	Excavation	3 days	Thu 5/8/25	Sat 5/10/25	
13	Drainage	6 days	Mon 5/12/25	Sat 5/17/25	
14	Tie-In Existing, Catch Basins	3 days	Mon 5/12/25	Wed 5/14/25	
15	Stormwater Swale	6 days	Mon 5/12/25	Sat 5/17/25	
16	Roads and Curbs	14 days	Fri 5/16/25	Thu 5/29/25	
17	Aggregate Layers	4 days	Fri 5/16/25	Mon 5/19/25	
18	Grading	4 days	Mon 5/19/25	Thu 5/22/25	
19	Concrete Pavement	5 days	Fri 5/23/25	Tue 5/27/25	
20	Quality Control	4 days	Mon 5/26/25	Thu 5/29/25	
21	Phase 2 - Exterior Lane Work	22 days	Tue 5/27/25	Fri 6/20/25	
22	Remove Existing Works	11 days	Tue 5/27/25	Fri 6/6/25	
23	Demolition	6 days	Tue 5/27/25	Sun 6/1/25	
24	Excavation	5 days	Mon 6/2/25	Fri 6/6/25	
25	Drainage	12 days	Fri 5/30/25	Wed 6/11/25	
26	Tie-In Existing, Catch Basins	11 days	Sat 5/31/25	Wed 6/11/25	
27	Roads and Curbs	13 days	Tue 6/3/25	Wed 6/18/25	
28	Aggregate Layers	6 days	Tue 6/3/25	Mon 6/9/25	
29	Grading	3 days	Sun 6/8/25	Tue 6/10/25	
30	Concrete Pavement	6 days	Wed 6/11/25	Wed 6/18/25	
31	Quality Control	3 days	Wed 6/18/25	Fri 6/20/25	
32	Phase 3 - Interior Lane Work	27 days	Mon 6/23/25	Thu 7/24/25	
33	Remove Existing Works	6 days	Mon 6/23/25	Sat 6/28/25	
34	Demolition	3 days	Mon 6/23/25	Wed 6/25/25	
35	Excavation	3 days	Thu 6/26/25	Sat 6/28/25	
36	Gateway Sign	5 days	Mon 6/30/25	Fri 7/4/25	
37	Concrete Footing, Cast-in-Place	1 day	Mon 6/30/25	Mon 6/30/25	
38	Sign Assembly	2 days	Thu 7/3/25	Fri 7/4/25	
39	Drainage	6 days	Mon 7/7/25	Sat 7/12/25	
40	Stormwater Management	6 days	Mon ///25	Sat //12/25	
41	Roads and Curbs	14 days	Tue 7/8/25	Wed 7/23/25	
42	Aggregate Layers	6 days	Tue 7/8/25	Sun //13/25	
43	Grading	4 days	Sun 7/13/25	Wed 7/16/25	
44	Concrete Pavement	4 days	Wed 7/16/25	Mon 7/21/25	
45		3 days	Mon 7/21/25	Wed 7/23/25	
46	Project Closeout	19 days	Mon 7/21/25	Wed 8/13/25	
47	Signage and Signalling	8 days	Won 7/21/25	Tue 7/29/25	
48		3 days		weu //23/25	_
49		3 days	FII //25/25	IVIOII //28/25	_
50		2 days	IVION //28/25	Tue 7/29/25	
51	Landscaping	10 days	Tue 7/22/25	Fri 8/1/25	
52	Final inspections, Quality	/ days	weu //30/25	Thu 8/7/25	_
53	Documentation	T2 gays	won 7/21/25	ι πu δ/ //25	
54	Site Demobilization	11 days	Wed 7/30/25	Wed 8/13/25	
55	Site Demobilization	11 days	Wed 7/30/25	Wed 8/13/25	



APPENDIX I

PROJECT CHARTER



Title	Chancellor Boulevard - East Mall Intersection Redesign		
Client	UBC Campus and Community Planning		
Project Manager	Capstone Consulting Limited		

Description

Capstone Consulting is redesigning the NW Marine Drive, East Mall, and Chancellor Boulevard intersection at UBC to align with the UBC Transportation Plan. The project aims to enhance safety, promote sustainability, and improve mobility for all users by incorporating traffic calming measures, bike lanes, and green infrastructure. A rainwater retention system will address stormwater management and prevent cliff erosion. Additionally, an eight-meter tall gateway sign, blending with campus architecture, will establish the intersection as a prominent campus entry point.

Business Case (Objective)	Scope (Deliverable)
Prioritize Active Travel Modes	 → Removal of existing works. → Tie-in existing utilities to stormwater solution. → Fill aggregate layers, ensuring proper grading and compaction. Pavement as per design drawings. → Signage, signalling, road markings as per design drawings.
Promote User Intuitivity and Safety	→ Signage, signalling, road markings as per design drawings.
Establish Intersection as Prominent Campus Entry	 → Cast concrete footing in place. → Assemble gateway structure as per design drawings.
Prepare Site for Major Storm Event, Prevent Erosion	 → Tie-in existing utilities to stormwater solution. → Upgrade existing green space to include bioswales, absorbent landscaping.

Key Milestone	Days Required	Scheduled Date	Actual Date
Phase 1 Complete	42	May 28, 2025	TBD
Phase 2 Complete	20	June 20, 2025	TBD
Phase 3 Complete	20	July 16, 2025	TBD
Project Complete	18	August 16, 2025	TBD



Original Contract Value		CA\$5,728,607.14	
Incidental	Stipulation		Value
Contingency	12.00%		CA\$687,432.86
Change Orders	Cost + 15.00% Overh	ead	TBD

Risk Assessment Matrix

	Insignificant 1	Minor 2	Significant 3	Major 4	Severe 5
1 Rare	1	2	3	4	5
2 Unlikely	2	4	6	8	10
3 Possible	3	6	9	12	15
4 Likely	4	8	12	16	20
5 Near Certain	5	10	15	20	25

Level	Low	Moderate	High	Critical
Score	1 - 5	6 - 11	12 - 18	19 - 25

Risk Summary

Risk	Score	Level
Regulatory Compliance Measures	20	Critical
Stakeholder Concerns and Consultation	15	High
Traffic Disruptions and Congestion	12	High
Utilities and Infrastructure Conflicts	8	Moderate
Budget Overruns and Construction Issues	6	Moderate

Risk Mitigation Strategies

Risk	Approach
Regulatory Compliance Measures	 Engage consultants early in the design and planning stages. Conduct a regulatory review to identify all applicable CoV and BC MOTI requirements.



	 Develop a compliance checklist and schedule regular audits throughout the project. Maintain communication with CoV and secure necessary approvals in advance.
Stakeholder Concerns and Consultation	 Initiate early and continuous stakeholder engagement through public meetings, surveys, and online feedback tools. Uphold UBC's Indigenous Strategic Plan (ISP)- see our Indigenous Consultation Plan Document all concerns and integrate feasible suggestions into the design and planning phases. Provide regular updates to stakeholders about project milestones and decisions.
Traffic Disruptions and Congestion	 Develop a detailed traffic management plan, including detours and clear signage for all road users. Schedule construction activities during off-peak hours or academic breaks to reduce impact. Coordinate with UBC CCP, emergency response, and Translink for smooth flow. Use digital platforms or signage to alert the public of closures and alternative routes in advance.
Utilities and Infrastructure Conflicts	 General contractor to conduct utility locates and subsurface scans before any excavation. Coordinate closely with utility companies to understand existing service lines and future plans. Allocate schedule float time in case of unexpected findings during construction. Engage our Erosion Sediment Control Plan (ESC) to protect existing infrastructure.
Budget Overruns and Construction Issues	 Use detailed cost estimation models and include contingency buffers (already at 12%). Monitor progress through regular financial and construction audits. Implement a change order approval system with oversight to manage scope creep. Pre-qualify contractors and suppliers to ensure performance and reliability.

APPENDIX J

COST ESTIMATE

CONSTRUCTION WORKS - INTERSECTION	ι	JNIT PRICE	UNIT	QUANTITY		COST
Concrete Walks, Curbs, & Gutters						
Extruded Concrete Curb - MMCD C6	\$	120.00	L.M.	500	\$	60,000.00
Concrete Sidewalk & Connector Walks (100mm Thickness per BBY-C101 & BBY-C124 to C128)	\$	140.00	S.M.	1200	\$	168,000.00
Concrete Wheelchair Letdown (Extra over)	\$	200.00	EACH	8	\$	1,600.00
Precast Concrete						
Roadside Barriers	\$	1,500.00	EACH	3	\$	4,500.00
Roadway Lighting			54.011		•	
Existing Pole Relocation with New Base and New LED Light	\$	3,000.00	EACH	4	\$	12,000.00
53mm RPVC Conduit c/w 3 #6 RW90 Cu. + 1 #8 RW90 Cu.	\$	350.00	L.M.	20	\$	7,000.00
Clearing and Grubbing	^	400.000.00	1.0	4	•	100.000.00
Clearing and Grubbing - All Locations	\$	100,000.00	L.S.	1	\$	100,000.00
Tree Removal (<=0.3m DBH Tree Trunk)	\$	1,000.00	EACH	1	\$	1,000.00
Tree Removal (>0.3m DBH Tree Trunk)	\$	2,500.00	EACH	1	\$	2,500.00
Shrub & Tree Preservation	•	00.00	LM	00	*	4 000 00
Tree Protection Fencing (BBY-L103)	\$	60.00	L.M.	80	\$	4,800.00
Reshaping Granutai Roaubeu	¢	4.00	C M	5005	•	00.000.00
Residuring	Ф	4.00	5.№.	5805	\$	23,220.00
Roadway Excavation, Embankment, & Compaction	•	05.00	0.14	5005	•	000 175 00
Removal and Disposal Offsite of Existing Concrete Flatwork	\$	35.00	S.M.	5805	\$ \$	203,175.00
Removal and Disposal Offsite of Existing Concrete Curbs	\$	35.00	L.M.	500	\$	17,500.00
Removal and Disposal Offsite of Existing Asphalt Flatwork	\$	35.00	S.M.	5805	\$	203,175.00
Common Excavation – Offsite Disposal	\$	85.00	C.M.	1300	\$	110,500.00
Over Excavation and Backfill	\$	150.00	C.M.	1300	\$	195,000.00
Imported Embankment Fill	\$	55.00	IONNE	1300	\$	/1,500.00
			0.11		+	440 100 000
Full Depth Milling (Maximum 75mm Deep)	\$	20.00	S.M.	5805	\$	116,100.00
Full Depth Milling (Maximum 175mm Deep)	\$	25.00	S.M.	5805	\$	145,125.00
Granular Subbase						
75mm Minus Crushed Granular Subbase	\$	60.00	TONNE	960	\$	57,600.00
Granular Base						
19mm Minus Crushed Granular Base	\$	90.00	TONNE	240	\$	21,600.00
Asphalt Tack Coat						
Asphalt Tack Coat	\$	1.00	S.M.	5805	\$	5,805.00
Hot-Mix Asphalt Concrete Paving						
Asphaltic Pavement (Chancellor Blvd) MMCD UC#1 - 50mm Thickness	\$	220.00	TONNE	730	\$	160,600.00
Asphaltic Pavement (Chancellor Blvd) MMCD LC#1 - 75mm Thickness	\$	220.00	TONNE	1080	\$	237,600.00
Asphaltic Pavement (Local Roads) MMCD UC#2 - 40mm Thickness	\$	220.00	TONNE	580	\$	127,600.00
Asphaltic Pavement (Local Roads) MMCD LC#1 - 45mm Thickness	\$	220.00	TONNE	650	\$	143,000.00
Asphalt Drainage Curb (50mm high, 175mm wide)	\$	40.00	L.M.	500	\$	20,000.00
Painted Pavement Markings						
Supply & Install Thermoplastic Pavement Markings	\$	21,000.00	L.S.	1	\$	21,000.00
Supply & Install Thermoplastic Bike Lane Markings	\$	175.00	EACH	12	\$	2,100.00
Install Traffic Signs c/w Base	\$	500.00	EACH	5	\$	2,500.00
Pedestrian Crosswalks	\$	1,000.00	EACH	4	\$	4,000.00
Top Soil & Finish Grading						
Imported Topsoil (150mm thickness)	\$	100.00	C.M.	18	\$	1,800.00
Sodding						
Sodding	\$	15.00	S.M.	120	\$	1,800.00
Planting of Trees, Shrubs, and Ground Covers						
Proposed Tree c/w Root Barrier	\$	1,000.00	EACH	2	\$	2,000.00
Storm Sewers						
Catch Basin Lead - 150mm SDR28 PVC	\$	500.00	L.M.	50	\$	25,000.00
Manhole and Catch Basins						
Type V Top Inlet Catch Basin (BBY-S110)	\$	1,700.00	EACH	1	\$	1,700.00
Type VA Side Inlet Catch Basin (BBY-S105 and BBY-S110)	\$	2,000.00	EACH	12	\$	24,000.00
Major Adjustment of Existing Manholes c/w New Self Levelling Frame	\$	2,500.00	EACH	8	\$	20,000.00
Removal & Disposal of Existing Catch Basins/ Lawn Basins and Capping Existing Lead	\$	400.00	EACH	12	\$	4,800.00
Traffic Signals						
Traffic Detector Loops	\$	2,500.00	EACH	4	\$	10,000.00
Installation of Traffic Lights	\$	500,000.00	L.S.	1	\$	500,000.00
Junction Box	\$	3,168.00	EACH	1	\$	3,168.00
CONSTRUCTION WORKS - GATEWAY SIGN						
Footing						
20M Bar	\$	25.00	L.S.	1	\$	25.00
10M Bar	\$	10.00	L.S.	1	\$	10.00
30MPa Concrete Column	\$	300.00	CU.M.	7.56	\$	2,268.00
Aluminum Letters	\$	500.00	S.M.	35.00	\$	17,500.00
Wood Slab	\$	2,000.00	L.S.	1	\$	2,000.00
30MPa Concrete Footing	\$	300.00	CU.M.	7.2	\$	2,160.00
CONSTRUCTION WORKS - RAINWATER RETENTION SYSTEM	Ŧ				Ť	_,
Waste Material (Excavate & Dispose Off-Site)	\$	151.20	CU.M.	3714.5	\$	561.632.40
Single Pipe (150mm - 3 -4 m depth)	\$	902.88	L.M	20	\$	18,057 60
Impermeable Fabric	\$	26 40	L.M	350	\$	9,240.00
Dimple Board - 3.5mx20m	Ψ \$	1 215 28	FACH	15	\$	18 229 20
Sodding & Tonsoil - 150mm	Ψ ¢	1,210.20	SM	1023 0	÷	10,220.20
Single CB - Side Inlet	φ \$	43.20 1/1 725 02	5.M. Fach	1023.9	Ψ \$	58 902 60
	φ	14,720.92		4	Ψ	00,900.08
Red Line Drawings - Storm	¢	1 000 00	1 9 1	î	¢	2 000 00
Red Line Drawings - Water	φ ¢	1 000 00	1.0.	2	Ψ ¢	2,000.00
neu Line Diawingo - Walei Dod Lino Drawingo - Sanitany	φ Φ	1,000.00	L.Ə.	2	ቀ ዋ	2,000.00
neu Line Drawings - Janitary Rod Line Drawings - Doade	φ Φ	1,000.00	L.Ə.	2	ቀ ዋ	2,000.00
neu Line Didwillgs - Rudus	\$	1,000.00	L.S.	2	Ф	∠,000.00

Red Line Drawings - Communications Conduit	\$ 1,000.00	L.S.	2	\$	2,000.00
Permits					
Development and Building Permits	\$ 800.00	L.S.	1	\$	800.00
Streets and Landscape Permit	\$ 820.00	L.S.	1	\$	820.00
Electrical Work Permits	\$ 820.00	L.S.	1	\$	820.00
Traffic Management Plan	\$ 800.00	L.S.	1	\$	800.00
Highway Use Permit	\$ 800.00	L.S.	1	\$	800.00
Project Planning					
Community Consultation Plan	\$ 5,000.00	L.S.	1	\$	5,000.00
LABOUR AND EQUIPMENT					
Project Director	\$ 1,200.00	L.D.	9	\$	10,800.00
Project Manager	\$ 1,100.00	L.D.	90	\$	99,000.00
Design Engineer	\$ 1,200.00	L.D.	90	\$	108,000.00
Project Coordinator	\$ 1,100.00	L.D.	235	\$	258,500.00
Superintendant	\$ 1,200.00	L.D.	235	\$	282,000.00
Foreman (x6)	\$ 700.00	L.D.	1410	\$	987,000.00
Design Engineer	\$ 800.00	L.D.	30	\$	24,000.00
Installation of Electrical	\$ 2,000.00	L.D.	5	\$	10,000.00
Tow Trucks	\$ 600.00	R.D.	235	\$	141,000.00
Tow Truck Driver	\$ 500.00	L.D.	235	\$	117,500.00
Excavator	\$ 560.00	R.D.	9	\$	5,040.00
Concrete Truck	\$ 600.00	R.D.	36	\$	21,600.00
Concrete Truck Operator	\$ 900.00	L.D.	36	\$	32,400.00
Crane	\$ 1,800.00	R.D.	2	\$	3,600.00
Crane Operator	\$ 1,000.00	L.D.	1	\$	1,000.00
Street Light Installation	\$ 1,600.00	L.D.	5	\$	8,000.00
Utility Relocation	\$ 2,000.00	L.D.	10	\$	20,000.00
Welder	\$ 1,500.00	L.D.	1	\$	1,500.00
			Total	\$ 5	5 728 606 36

 Total
 \$ 5,728,606.36

 Contingency
 12%

 FINAL TOTAL
 \$ 6,416,039.12

APPENDIX K

SERVICE LIFE MAINTENANCE PLAN

Surface Maintenance Activities			
Task Description	Frequency of the Task		Cost
Patching/sealing paved surfaces with	Eveny 3-5 years	\$	31 500 00
chemical compounds/treatment		Ψ	01,000.00
Sweeping and disposal of surface debris	Monthly – Quarterly	\$	42,000.00
Grading and shoulder repair	Annually	\$	72,000.00
Curb maintenance (curb painting, repaving,	Every 2-5 years	\$	280,000.00
etc.)			
Painting maintenance (crosswalks, bike	Every 1-2 years	\$	48,000.00
	Even 5 years	¢	6 000 00
Benlacement/reinstallation of surface	Every 5 years	φ	0,000.00
reflectors and delineators	Every 3-5 years	\$	1,000.00
Drainage Maintenance Activities			
Task Description	Frequency of the Task		Cost
Bemoval of obstructions and debris from			0000
catch basins	Spring/Fall	\$	7,200.00
Repair and reinstallation of storm sewer	F 40.00		40.000.00
components	Every 10-30 years	\$	10,000.00
Traffic Signal Maintenance Activities			
Task Description	Frequency of the Task		Cost
Inspection of signal controllers	Annually	\$	75,000.00
Inspection of signal lights and bulb			
replacements	Annually	\$	40,000.00
Maintenance of signal poles	Every 5 years	\$	25,000.00
Sign cleaning and replacement	Every 2-3 years	\$	30,000.00
Vegetation Maintenance Activities			
Task Description	Frequency of the Task		Cost
Shoulder/boulevard mowing/bush whacking	Monthly	\$	96,000.00
Sight line clearing	Annually	\$	96,000.00
Winter Maintenance Activities			
Task Description	Frequency of the Task		Cost
Road sanding/salting, plowing, and snow	A		10.000.00
removal	Annually	Þ	12,000.00
Clearing out snow from high foot traffic areas	Annually	\$	4,000.00
Spring cleaning of winter aggregates	Annually	\$	10,000.00
Gateway Sign Maintenance Activities		,	
Task Description	Frequency of the Task		Cost
Inspection of overall sign condition	Annually	\$	7,000.00
Cleaning of aluminum lettering	Every 1-2 years	\$	8,000.00
Re-coating of aluminum letters	Every 10-15 years	\$	1,000.00
Wood slab staining/sealing	Every 3-5 years	\$	2,000.00
Replacement of damaged wooden	Event 10, 20 years	¢	2 000 00
components		9	2,000.00
Concrete crack repair/sealing	Every 5-10 years	\$	3,200.00
Structural integrity check	Every 5 years	\$	2,000.00
Rainwater Swale and Raingarden Maintenar	ice Activities		
Task Description	Frequency of the Task		Cost
Pre-treatment sediment treatment basin	Quarterly or after major storm events	\$	80,000.00
Cleaning Monodatation management	Monthly or ofter major storm events	¢	06 000 00
Inspection for sediment huildun erosion and		φ	30,000.00
channeling	Monthly or after major storm events	\$	36,000.00
Removal of trash and debris	Monthly or after major storm events	\$	36.000.00
Monitoring for water pollutants in swale	Every 2-3 years	\$	12.000.00
Soil and vegetation overhaul	Every 3-5 years	\$	24.500.00
	Total (12% Contingency)	\$	1 339 000 00



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UBC CAMPUS AND COMMUNITY PLANNING

6200 UNIVERSITY BOULEVARD, VANCOUVER BC, V6T 1Z4 PH. 604-221-2570

PROJECT:

CHANCELLOR BOULEVARD & EAST MALL INTERSECTION REDESIGN UNIVERSITY ENDOWMENT LANDS

c	OFFSITE DRAWING INDEX
DRAWING NO.	DRAWING TITLE
01	COVER SHEET
02	GENERAL NOTES
03	KEY PLAN 1
04	KEY PLAN 2
05	GRADING PLAN
06	PROFILE VIEW
07	SAMPLE LINE PLAN VIEW
08	A1 SECTION VIEW (1)
09	A1 SECTION VIEW (2)
10	A2 SECTION VIEW (1)
11	A2 SECTION VIEW (2)
12	SIGNAGE AND PAINT PLAN
13	RAINWATER MANAGEMENT DETAILS
14	GATEWAY SIGN DIMENSIONS
15	GATEWAY SIGN DETAILS
16	CONSTRUCTION PHASING
17	TMP PHASE 1 ROAD CLOSURE
18	TMP PHASE 2 INTERSECTION
19	TMP PHASE 2 MIDBLOCK
20	TMP PHASE 3 TYPICAL
21	STANDARD ROADWORKS DETAILS
22	STANDARD UTILITIES & GI DETAILS
23	STANDARD ELECTRICAL DETAILS (1)
24	STANDARD ELECTRICAL DETAILS (2)

A. GENERAL NOTES

- ALL CONSTRUCTION TO BE COMPLETED IN ACCORDANCE WITH THE LATEST REQUIREMENTS AND SPECIFICATIONS FROM THE CITY OF VANCOUVER ENGINEERING DESIGN MANUAL, STANDARD DETAIL DRAWINGS (SDD), THE CONSTRUCTION SPECIFICATIONS, AND THE APPLICABLE MASTER MUNICIPAL CONSTRUCTION DOCUMENT (MMCD) SPECIFICATIONS UNLESS OTHERWISE NOTED. 1.
- 2. CONTACT BC 1 CALL TO OBTAIN UTILITY LOCATION INFORMATION 2 WEEKS PRIOR TO CONSTRUCTION.
- 3. THE CONTRACTOR IS TO NOTIFY THE ENGINEER 48 HRS. PRIOR TO CONSTRUCTION TO VERIFY THAT THEY HAVE THE LATEST DRAWINGS ISSUED FOR CONSTRUCTION.
- 4. THE CONTRACTOR IS TO MAINTAIN ON-SITE COPIES OF THE MOST CURRENT SET OF DESIGN DRAWINGS AT ALL TIMES, AND MARK UP IN RED ANY FIELD REVISIONS OR APPROVED DEVIATIONS FROM THE DESIGN, THIS REDLINED SET IS TO BE AVAILABLE ONSITE FOR THE CIVIL ENGINEER TO REVIEW AT ALL TIMES.
- 5. ANY REVISIONS TO THESE DRAWINGS MUST BE APPROVED BY THE ENGINEER, WHO SHALL REVIEW ANY CHANGES WITH THE CITY OF VANCOUVER ENGINEER.
- ALL LOCATIONS AND ELEVATIONS OF EXISTING UNDERGROUND UTILITIES HAVE BEEN DETERMINED FROM CITY OF VANCOUVER GEOGRAPHIC INFORMATION SYSTEM (GIS) AND THE ENGINEER CANNOT BE HELD RESPONSIBLE FOR THEIR ACCURACY. THE CONTRACTOR SHALL FIELD VERIFY AND EXPOSE THE LOCATION AND ELEVATION OF ALL EXISTING SERVICES AND THE-IN POINTS PRIOR TO CONSTRUCTION AND TO IMMEDIATELY NOTIFY THE ENGINEER OF ANY 6. DISCREPANCIES, CONFLICTS OR OMISSIONS.
- DIMENSIONS, GRADES, COORDINATES, AND DETAILS OF THE DESIGN ARE TO BE CONFIRMED IN THE FIELD PRIOR TO CONSTRUCTION. IMMEDIATELY NOTIFY THE ENGINEER OF ANY DISCREPANCIES, CONFLICTS OR CMISSIONS.
- 8. ALL SITEWORK, TRENCHING, EXCAVATION, BACKFILL, ROADWORKS AND CONCRETE WORKS ARE TO BE SUPPLED AND CONSTRUCTED IN ACCORDANCE WITH CITY OF VANCOUVER STANDARDS. IN CASES WHERE NO CITY OF VANCOUVER STANDARD EXISTS, THEN MMCD STANDARDS ARE TO BE ADHERED TO.
- 9. THE CONTRACTOR SHALL OBTAIN ALL PERMITS AND LICENSES PRIOR TO CONSTRUCTION AND ENSURE THAT ALL APPROVALS REQUIRED FOR THE PROPOSED WORK HAVE BEEN OBTAINED. THE CONTRACTOR IS TO CHECK WITH FORTIS BC, B.C. HYDRO AND TELLIS PRIOR TO COMMENCING WORK TO VERIFY THE LOCATIONS AND INVERTS OF THESE SERVICES.
- 10. ALL MATERIALS TESTING MUST BE DONE IN ACCORDANCE WITH THE CITY OF VANCOUVER SPECIFICATIONS. TESTING TO BE CARRIED OUT BY A QUALIFIED MATERIALS TESTING FIRM AND PAID FOR BY THE CONTRACTOR. THE ENGINEER AND THE CITY ENGINEER SHALL RECEIVE COPIES OF ALL TEST RESULTS FROM THE CONTRACTOR.
- 11. TRAFFIC CONTROL (SIGNS, BARRICADES, FLAG PERSONS) IS TO BE MAINTAINED AT ALL TIMES WHILE WORKING ON MUNICIPAL RIGHT-OF-WAYS. A TRAFFIC OBSTRUCTION PERMIT MUST BE OBTAINED FROM THE CITY'S ENGINEERING DEPARTMENT PRIOR TO PARTIAL OR FULL ROAD CLOSURES.
- 12. TRAFFIC CONTROL TO CONFORM TO THE LATEST EDITION OF THE "TRAFFIC CONTROL MANUAL FOR WORK ON ROADWAYS" AS PUBLISHED BY THE MINISTRY OF TRANSPORTATION AND HIGHWAYS.
- 13. ANY MATERIAL SUBSTITUTION MUST BE APPROVED BY THE ENGINEER AND THE CITY ENGINEER.
- 14. THE CONTRACTOR SHALL USE EXTREME CARE WHEN WORKING NEAR EXISTING SERVICES AND INFRASTRUCTURE. ALL SERVICES AND INFRASTRUCTURE DISTURBED DURING CONSTRUCTION SHALL BE RESTORED TO ORIGINAL OR BETTER CONDITION AND TO THE SATISFACTION OF THE CITY OF VANCUVER OR OTHER APPROVING AGENCIES.
- 15. THE CONTRACTOR WILL BE HELD RESPONSIBLE FOR REPAIR OF ANY DAMAGE CAUSED TO EXISTING STREET OR SERVICES BY CONSTRUCTION EQUIPMENT AND/OR TRUCKS HAULING MATERIAL TO AND FROM THE SITE THIS MAY INCLUDE DAILY CLEANING OR SWEEPING EXISTING ROADS OF DIRT AND DEBRIS CAUSED BY THE CONSTRUCTION ACTIVITY.
- 16. OPEN TRENCH OPERATIONS IN EXISTING PAVEMENT SHALL BE VERTICAL. THE CONTRACTOR SHALL CONFIRM TRENCH COVERING REQUIREMENTS DURING CONSTRUCTION WITH THE CITY, PRIOR TO CONSTRUCTION. THE CONTRACTOR WILL BE HELD RESPONSIBLE TO PREVENT ANY HAZARD FOR PEDESTRIANS AND VEHICLES. ALL PAVEMENT, BOULEVARDS, ETC. ARE TO BE RESTORED TO ORIGINAL CONDITIONS WHERE NO IMPROVEMENT IS PROPOSED UNDER THIS
- 17. ALL SURVEY MONUMENTS, POSTS, OR IRON PINS MUST BE PROTECTED AND ANY DAMAGE SHALL BE REPAIRED PRIOR TO THE COMPLETION OF CONSTRUCTION AT THE CONTRACTOR'S EXPENSE.
- 18. ALL EXISTING IMPROVEMENTS ARE TO BE RESTORED TO THE SATISFACTION OF THE CITY OF VANCOUVER.
- 19. FIGURED DIMENSIONS SHALL GOVERN OVER SCALED DIMENSION.
- 20. FOR ANY QUESTIONS, PROPOSED CHANGES, OR CONSTRUCTION ISSUES, CONTACT
- 21. FOR REMOVAL OF STREET FURNITURE, CONTACT STREET FURNITURE COORDINATOR AT street.furniture@vancouver.ca A NINIMUM OF 4 WEEKS PRIOR TO CONSTRUCTION.

B. ROADWORK NOTES

- OFFSITE ROADWORKS TO BE CONSTRUCTED IN ACCORDANCE WITH THE CITY OF VANCOUVER DESIGN GUIDELINES, CONSTRUCTION SPECIFICATIONS, AND STANDARD DETAIL DRAWINGS. 1.
- 2. ALL MANHOLE COVERS, VALVE COVERS, CATCHBASIN RIMS, AND LIDS OF ANY OTHER STRUCTURE OR UTILITY ARE TO BE ADJUSTED TO SUIT FINAL GRADES.
- 3. ALL LOOSE AND ORGANIC MATERIALS ARE TO BE EXCAVATED AND REMOVED FROM ROADWAY AS APPROVED BY THE ENGINEER PRIOR TO PLACING OF ANY ROAD GRAVELS.
- 4. ALL SUBGRADES TO BE COMPACTED TO 95% MODIFIED PROCTOR DENSITY, AS WELL AS ALL GRANULAR SUBBASE AND BASE MATERIALS.

- 5. CHANGES IN GRADE ARE TO BE FORMED WITH SMOOTH CURVES.
- 6. TESTING OF ROAD MATERIALS AND COMPACTION TO BE COMPLETED IN ACCORDANCE WITH THE CITY'S CONSTRUCTION SPECIFICATIONS.
- 7. CONTACT MINIMUM 2 WEEKS IN ADVANCE OF ANY SIDEWALK OR MEDIAN CONCRETE WORK TO OBTAIN SIGN-POLE SLEEVES/BASES AND CONFIRM LOCATIONS.
- 8. FOR ALL CURB RAMPS, CONTACT IN CROSSWALK LOCATIONS. TO HAVE CREWS GHOST
- SCORING PATTERNS FOR CURB RAMPS SHALL CONFORM TO CITY STANDARD DETAIL DRAWINGS C8.1, C8.2, C8.3, AND C8.4 AND INDICATE DIRECTION OF TRAVEL IN TO CROSSWALK.
- 10. INSTALL ALL SIGNAGE, PARKING METER, AND BUS ID SLEEVES AS PER CITY STANDARD DETAIL DRAWINGS C19.1, C19.2, AND C19.3. CONTACT SIGN FLAN AND LOCATIONS.
- 11. TIE-INS TO EXISTING PAVEMENT SHALL BE MADE BY CUTTING BACK THE EXISTING PAVEMENT TO SOUND MATERIAL AS NECESSARY TO PRODUCE A NEAT VERTICAL FACE WITH A STRUCHT EDGE, PRIOR TO PLACING HOT-MIX ASPHALTIC CONCRETE, EXPOSED PAVEMENT FACES SHALL BE PAINTED WITH LIQUID ASPHALT AND HEATED TO 65 DEGREES CELSUS. THE FINISHED PAVEMENT SURFACE SHALL BLEND IN SMOOTHLY WITH EXISTING PAVEMENT, ALL PAVEMENTS, BOULEVARDS, ETC. ARE TO BE RESTORED TO ORIGINAL CONDITIONS WHERE NO IMPROVEMENT IS PROPORT UNDER THIS CONTRACT. IS PROPOSED UNDER THIS CONTRACT.
- 12. LOCATIONS OF DRIVEWAYS, WHEELCHAIR RAMPS, ETC. SHALL BE CONFIRMED IN THE FIELD PRIOR TO CONSTRUCTION OF ANY PROPOSED CONCRETE CURB AND GUTTER AND SIDEWALK.
- 13. PLACEMENT OF ASPHALTIC CONCRETE AND PORTLAND CEMENT CONCRETE SHALL BE UNDERTAKEN ONLY WHEN WEATHER CONDITIONS ARE IN CONFORMANCE WITH THE CITY OF VANCOUVER SPECIFICATIONS.
- 14. THE ROAD BASE SHALL EXTEND A MINIMUM OF 0.3m BEYOND THE BACK OF CURB AND GUTTER, WHICHEVER IS GREATER.
- 15. TESTING WILL BE REQUIRED TO CONFIRM THE SUITABILITY OF THE ROAD STRUCTURE PRIOR TO PAVING IN ACCORDANCE WITH CITY OF VANCOUVER CONSTRUCTION SPECIFICATIONS (2019) AND THE LATEST VERSION OF MMCD.
- CONTRACTOR TO VERIFY FORM WORK FOR SIDEWALK WITH THE INSPECTOR FOR THE CITY OF VANCOUVER. FORM WORK TO BE APPROVED BY Cov PRIOR TO POURING SIDEWALK.
- 17. CONTRACTOR TO FIELD VERIFY THAT PROPOSED GUTTER PAN TIE-IN MEETS WITH EXISTING ROADWAY CROSSFALL.
- 18. DAMAGES TO ANY EXISTING Cov INFRASTRUCTURE ADJACENT TO THE SITE DURING CONSTRUCTION IS TO BE REPLACED TO CURRENT COV STANDARDS, REPLACEMENT TO BE DETERMINED AT THE SOLE DISCRETION OF THE GENERAL MANAGER OF ENGINEERING SERVICES.

C. STREET TREES:

- ALL TREE INSTALLATIONS TO BE COMPLETED IN ACCORDANCE WITH THE CITY OF VANCOUVER ENGINEERING DESIGN MANUAL, STANDARD DETAIL DRAWINGS (SDD) AND THE CONSTRUCTION SPECIFICATIONS, UNLESS OTHERWISE NOTED.
- 17. PRIOR TO CONSTRUCTION, PROTECT ALL EXISTING TREES WITHIN THE CONSTRUCTION AREA WITH TREE PROTECTION FENCING AS OUTLINED IN THE COV STANDARD DETAIL DRAWINGS.
- 18. PRIOR TO CONSTRUCTION, IDENTIFY ANY POTENTIAL TREE IMPACTS AND CONTACT CITY OF VANCOUVER PARKS FOR FURTHER DIRECTION.
- 19. ALL ELECTRICAL CONDUIT TO BE LOCATED AT BACK OF CURB TO ALLOW FOR TREE PLANTING IN BOULEVARD UNLESS OTHERWISE INSTRUCTED.

D. SANITARY SEWER AND STORM SEWER NOTES

- TWO WEEKS PRIOR TO THE COMMENCEMENT OF ANY GROUND DISTURBANCE WORKS ON THE SITE, THE CONITRACTOR SHALL PRE-LOCATE (EM/GPR OR APPROVED EQUIVALENT), HYDRO-VAC (WHERE REQUIRED), AND MARK THE LOCATION OF ALL EXISTING UNDERGROUND UTILITIES ACROSS THE EXTENT OF THE SITE. THE PRE-LOCATION WORKS SHALL BE PERFORMED BY A THIRD PARTY SERVICE LOCATOR. ANY DISCREPANCIES WITH THE INFORMATION SHOWN THAT MAY ALTER THE DESIGN SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER
- 2. HAND DIG WHEN EXCAVATING WITHIN 0.5m OF EXISTING UTILITIES.
- 3. A MINIMUM OF 1.0m COVER IS REQUIRED OVER STORM AND SANITARY PIPING, THE ENGINEER IS TO BE NOTIFIED OF ANY CHANGES PRIOR TO INSTALLATION.
- 4. FOR STREET RESTORATION SEE LATEST CoV STANDARD DRAWING G5.1.
- WHERE SEWER MAIN IS ABANDONED, USE CONTROLLED DENSITY FILL (CDF) AS REQUIRED BY CONTRACT SPECIFICATIONS.
- 6. ALL STORM AND SANITARY SEWER MAIN DEFLECTIONS ARE TO CONFORM TO HALF OF THE MANUFACTURER'S SPECIFICATIONS.
- STORM AND SANITARY SEWERS ARE TO BE PVC SDR 28 FOR PIPES LESS THAN 200mm DIAMETER AND PVC SDR 35 FOR 200mm DIAMETER PIPE OR GREATER UNLESS OTHERWISE NOTED.
- 8. ALL MH BENCHING FOR STORM AND SANITARY SEWERS TO BE FULL HEIGHT BENCHING
- FOR CLOSED RAMP (C/R) AND OPEN RAMP (O/R) MANHOLE DROP CONNECTIONS, REFER TO LATEST CAN STANDARD DRAWING DWG S3.1. CONTRACTOR TO SUBMIT SHOP DRAWINGS FOR MANHOLES FOR APPROVAL PRIOR TO CONSTRUCTION.
- 10. ALL WYES ARE TO BE MANUFACTURED.
- 11. THE CONTRACTOR MUST PRESSURE TEST AND FLUSH ALL NEW SEWER LINES PRIOR TO TIE-IN PROCEDURES. TESTS ARE TO BE PERFORMED AS PER CITY OF VANCOUVER CONSTRUCTION SPECIFICATIONS. THE CONTRACTOR IS TO PROVIDE THE ENGINEER AND THE CITY ENGINEER WITH THE TEST RESULTS.

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UBC CAMPUS AND COMMUNITY PLANNING

6200 UNIVERSITY BOULEVARD, VANCOUVER BC, V6T 1 Z4 PH. 604-221-2570

CLIENT

PROJECT

CHANCELLOR BOULEVARD & EAST MALL INTERSECTION REDESIGN UNIVERSITY ENDOWMENT LANDS

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12. THE CONTRACTOR IS TO REVIEW THE GEOTECHNICAL REPORT AND DETERMINE IF DEWATERING MEASURES ARE NECESSARY DURING TRENCHING OPERATIONS. THE CONTRACTOR IS TO PROVIDE A COST FOR NECESSARY DEWATERING MEASURES IN THE CONTRACT BID.

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13. SANITARY SEWERS ARE TO BE CONSTRUCTED WITH SEALED JOINTS UNLESS NOTED OTHERWISE.

14. SUPPORT OF EXISTING WATER MAINS IS REQUIRED AT ALL CROSSINGS. CONTRACTOR TO SUBMIT DETAIL OF SUPPORT DESIGN TO CITY OF VANCOUVER FOR APPROVAL AT LEAST 2 WEEKS PRIOR TO COMMENCEMENT OF CONSTRUCTION. WATER MAIN SUPPORT DESIGN TO BE SIGNED AND SEALED BY STRUCTURAL ENGINEER.

ALL WATER CROSSINGS TO BE WITNESSED BY WW REPRESENTATIVE. CONTRACTOR TO CONTACT COV WATER REPRESENTATIVE TO BE ON SITE DURING WATER CROSSINGS, AT LEAST 1 WEEK PRIOR TO COMMENCEMENT OF CONSTRUCTION.

E. SEWER SERVICE CONNECTION NOTES

RECONNECT ALL SERVICE CONNECTIONS TO NEW PROPOSED MAINS: SANITARY CONNECTIONS TO SANITARY MAIN, STORM CONNECTIONS TO STORM MAINS. COMBINED CONNECTIONS TO STORM MAIN. PER Cov DESIGN STANDARDS.

2. LEAVE 100 COM WYES FOR ALL LOTS. MATCH EXISTING CONNECTION DIAMETER IF LARGER.

3. EXISTING LOT SEWER SERVICE CONNECTIONS ASSUMED TO BE 100# COMBINED. MATCH EXISTING DIAMETER IF LARGER.

4. TIE EXISTING CB(S) TO PROPOSED STORM SEWER.

5. ALL LOTS AND UTILITY MH(S) HAVE EXISTING SERVICE CONNECTIONS EVEN IF NO CONNECTIONS INFORMATION OR LOCATIONS ARE INDICATED ON THE DRAWING.

REFER TO COV STD DETAIL DWG S7.1 AND S7.2 FOR SERVICE CONNECTION TIE-IN REQUIREMENTS.

ALL EXISTING ACTIVE SERVICE CONNECTIONS ARE TO BE MAINTAINED IN FUNCTION DURING CONSTRUCTION, UNLESS DIRECTED BY THE CITY OF VANCOUVER OTHERWISE.

"RELAY" OR "RE-CONNECT" EX. SERVICE CONNECTIONS, CB LEADS AND UTILITY DRAINS AS APPROPRIATE PER CURRENT VERSION OF THE CITY OF VANCOUVER STANDARD DETAIL DRAWINGS. "RELAY"= DIG BACK (OUTSIDE MAINLINE TRENCH) TO INTERCEPT EXISTING SERVICE AND LAY EXTENSION BETWEEN EX. AND PROP. SEWER ALIGNMENTS. "RECONNECT"= CUT SERVICE AND TRANSFER TO TO NEW SEWER (WITHIN MAINLINE TRENCH).

F. ENVIRONMENTAL:

ALL WORKS TO BE IN COMPLIANCE WITH CITY OF VANCOUVER, BC MINISTRY OF ENVIRONMENT AND FEDERAL FISHERIES REQUIREMENTS.

2. IMMEDIATELY NOTIFY THE ENGINEER OF ANY SUSPECTED ARCHAEOLOGICAL MATERIALS UNCOVERED DURING EXCAVATION.

3. PROVIDE DITCHING, SILT FENCING, AND CATCHBASIN SEDIMENT TRAPS. CONTAINMENT FACILITIES ARE REQUIRED TO PREVENT DISCHARGE OF SEDIMENT FROM WORK AREA.

4. ALL TEMPORARY FILL SLOPES AND STOCK PILES TO BE PROTECTED FROM WEATHER EROSION.

5. ALL EXPOSED SLOPES TO BE PROTECTED FROM WEATHER EROSION AND SEEDED AS SOON AS PRACTICABLE.

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<u>LEGEND</u>

PROPOSED CURB & GUTTER PROPOSED ROLLOVER CURB & GUTTER PROPOSED FLUSH CONCRETE STRIP PROPOSED CONCRETE SIDEWALK PROPOSED MULTI-USE PATHWAY PROPOSED CONCRETE BOULEVARD PROPOSED GRASS BOULEVARD PROPOSED GREEN INFRASTRUCTURE PROPOSED BIKE CROSSING PROPOSED WAYFINDING STRIP PROPOSED BIKE LANE SYMBOL $(\uparrow \uparrow)$ PROPOSED BIKE LANE PAINT PROP. YELLOW PAVEMENT MARKINGS EXIS. STORM SEWER __ n ___ __ __ __ __ PROP. CB LEAD EXIS. STORM MH PROP. / EXIS. CB EXIS. CURB AND GUTTER EXIS. SIDEWALK $\bigcirc \Rightarrow \stackrel{\land}{\rightarrow} \stackrel{\land}{\rightarrow} \stackrel{\frown}{\rightarrow}$ PROP. TRAFFIC LIGHT PROP. TRAFFIC PUSH BUTTON PROP STREETLIGHT PROP. U/G DETECTOR LOOPS

NOTES:

1. SEE DWG 21 - 24 FOR ALL STANDARD DETAILS.

2. SEE DWG 05 – 11 FOR GRADING DESIGN AND ROAD CROSS SECTIONS.

3. THE LOCATION OF EXISTING UNDERGROUND UTILITIES ARE SHOWN IN AN APPROXIMATE WAY. THE CONTRACTOR SHALL DETERMINE THE EXACT LOCATION OF ALL EXISTING UTILITIES BEFORE COMMENCING WORK.

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<u>LEGEND</u>

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PROPOSED ROLLOVER CURB & GUTTER	
PROPOSED FLUSH CONCRETE STRIP	
PROPOSED CONCRETE SIDEWALK	
PROPOSED MULTI-USE PATHWAY	
PROPOSED CONCRETE BOULEVARD	
PROPOSED GRASS BOULEVARD	* *
PROPOSED GREEN INFRASTRUCTURE	* *
PROPOSED BIKE CROSSING	
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NOTES:

- 1. SEE DWG 21 24 FOR ALL STANDARD DETAILS.
- 2. SEE DWG 05 11 FOR GRADING DESIGN AND ROAD CROSS SECTIONS.

3. THE LOCATION OF EXISTING UNDERGROUND UTILITIES ARE SHOWN IN AN APPROXIMATE WAY. THE CONTRACTOR SHALL DETERMINE THE EXACT LOCATION OF ALL EXISTING UTILITIES BEFORE COMMENCING WORK.

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UBC CAMPUS AND COMMUNITY PLANNING 6200 UNIVERSITY BOULEVARD, VANCOUVER BC, V6T 1Z4 PH. 604-221-2570

CLIENT:

PROJECT: CHANCELLOR BOULEVARD & EAST MALL INTERSECTION REDESIGN UNIVERSITY ENDOWMENT LANDS

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01 A1 SECTION VIEW (2) TF AZ APR 4 2025	ULTING CHANCELLOR BOULEVARD & EAST MALL INTERSECTION REDESIC	SCALE : 1:750 DRAWING DATE: APRIL 2025
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CLIENT:

UBC CAMPUS AND COMMUNITY PLANNING 6200 UNIVERSITY BOULEVARD, VANCOUVER BC, V6T 1Z4 PH: 604-221-2570

CHANCELLOR BOULEVARD & FAST MALL INTERSECT

CHANCELLOR BOULEVARD & EAST MALL INTERSECTION REDESIGN UNIVERSITY ENDOWMENT LANDS

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CLIENT: UBC CAMPUS AND COMMUNITY PLANNING 6200 UNIVERSITY BOULEVARD, VANCOUVER BC, V6T 1Z4 PH. 604-221-2570

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he location of existing nderground utilities are shown i an approximate way only & tave not been independently arfifed by the owner or its presentative. The contractor not determine the exact lad determine the exact lad off and the stating utilities afore commencing work, and grees to be fully responsible r any and all damages which	TITLE: A2 SECTION VIEW (2)	DESIGN: ALL CHEC	CK: AZ
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DETAIL A LEGEND
NOOD MULCH
NG LEVEL
TO ENSURE A MAX. 200mm DROP FROM SIDEWALK CURB
BASE COMPACTED TO 95% MPMDD
N SOIL FOR STORAGE
ABLE FABRIC FOR RETENTION ELEMENT. FABRIC PER CITY STANDARDS
n CLEAR CRUSH AGGREGATE
RESERVOIR COMPRISED OF ROUND RIVER ROCK STONE. 90-120mm
N INTO NATIVE SOIL
ARIFICATION IN NATIVE SOIL
COVERED CURB INLET AND PRETREATMENT SEDIMENT BASIN. SEE DETAIL A.
FLOW SUBDRAIN LAWNBASIN. SEE DETAIL C
CB LEAD TO TIE INTO EXIS. STM @ 2%

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	DETAIL B AND SECTION A-A LEGEND
1)	NEW ROAD CURB WITH BASE THICKENED BY 15mm
2)	CURB INLET WITH MODIFIED GUTTER PAN
3)	BOTTOM OF WEIR NOTCH
4)	VEGETATED SLOPE BEYOND INLET AREA DEPENDENT ON SITE CONDITIONS
5)	90-120mm ROUND RIVER ROCK STONE
6)	CIP PRETREATMENT SEDIMENT BASIN TO PREVENT BIORETENTION MEDIA
	EROSION AND REDUCE SEDIMENT ENTERING INTO GI. SEE DETAIL B
7)	COMPACTED GRANULAR BASE. REFER TO COV STD DWG FOR DETAILS
8)	STRAIGHT STEEL INLET COVER

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PLAN VIEW

<u>NOTES:</u> 1. ALL CONCRETE SHOWN WITH 28-DAY COMPRESSIVE STRENGTH OF 30MPa 2. ALL STEEL REINFORCEMENT BARS HAVE A YIELD STRENGTH OF 400MPa 3. ALL DIMENSIONS SHOWN IN MILLIMETERS 4. PROVIDE MINIMUM 30MM CLEAR COVER 5. FOR GENERAL NOTES REFER TO NOTES PAGE

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REV. NO. DESCRIPTION DR DATE 01 CONSTRUCTION PHASING PT PT MAR27 2025	CLIENT: UBC CAMPUS AND COMMUNITY PLANNING S200 UNIVERSITY BOULEVARD, VANCOUVER BC, V6T 124 PH. 604-221-2570 PROJECT: CHANCELLOR BOULEVARD & EAST MALL INTERSECTION REDESIGN UNIVERSITY ENDOWMENT LANDS	The location of existing underground utilities are shown in an approximate way only & hove not been independently verified by the owner or its representative. The contractor shall determine the exact before commencing work, and agrees to be fully responsible for any and all damages which might be occasioned by the contractor's failure to exactly locate and preserve any and all underground utilities.	DINSTRUCTION PHASING	SN: PT CHECK: AZ PT DRAWING DATE: APRIL 2025 SHEET NO. REV. 6 OF 24 01
 Phase 1 Phase 2 Phase 3 Site Trailer 		ONA DRIVE		
PACIFIC SPIRIT PARK		CHANCELLOR BLVD		



PACIFIC SPIRIT PARK		VORK ACTIVITY AREA	81 ¹⁰	
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REV. NO. DESCRIPTION DR DATE 01 TMP PHASE 2 MIDBLOCK PT PT APR 6 2025	UBC CAMPUS AND COMMUNITY PLANNING 6200 UNIVERSITY BOULEVARD, VANCOUVER BC, V6T 124 PH. 604-221-2570 PROJECT: CHANCELLOR BOULEVARD & EAST MALL INTERSECTION REDESIGN UNIVERSITY ENDOWMENT LANDS	In an approximate way only its have not been independently verified by the owner or its representative. The contractor shall determine the sout before commencing work, and agrees to be fully responsible for any and all damages which might be occessioned by the contractor's failure to exactly locate and preserve any and all underground utilities.	TMP PHASE 2 MIDBLOCK SCALE : 1:750 DRAWING NO. 19	PT DRAWING DATE: APRIL 2025 SHEET NO. REV. 19 OF 24 01



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UBC CAMPUS AND COMMUNITY PLANNING 6200 UNIVERSITY BOULEVARD, VANCOUVER BC, V6T 1Z4 PH. 604-221-2570

PROJECT: CHANCELLOR BOULEVARD & EAST MALL INTERSECTION REDESIGN UNIVERSITY ENDOWMENT LANDS The location of salat underground utilities in an approximate we have not been indepverified by the arms representative. The o shall determine the location of all ealth before commencing agrees to be fully re for any and all dam might be occusioned contractor's foilure to locate and preserve underground utilities.

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REV. NO. DESCRIPTION 01 STANDARD UTILITIES AND GI DETAILS

PROJECT: CHANCELLOR BOULEVARD & EAST MALL INTERSECTION REDESIGN UNIVERSITY ENDOWMENT LANDS The location of solal underground utilities in on approximate w have not been indep weiffied by the owne representative. The c about determinie the location of all eated before commencing agreese to bo difficult might be calling might be calling locate and preserve underground utilities.

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REV. NO. DESCRIPTION

01 STANDARD ELECTRICAL DETAILS (2)



APSTONE ONSULTING

DR DATE CM APR 7 2025

CLIENT:

PROJECT:

UBC CAMPUS AND COMMUNITY PLANNING	
6200 UNIVERSI'Y BOULEVARD, VANCOUVER BC, V6T 1 Z4 PH. 604-221-2570	

CHANCELLOR BOULEVARD & EAST MALL INTERSECTION REDESIGN UNIVERSITY ENDOWMENT LANDS The location of sulat underground utilities in an approximate we have not been indep verified by the owner representative. The o location of all estative before commencing agrees to be fully retor any and all dam might be occurrently contractor follows to locate and preserve underground utilities.

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